

CONSULTANTS IN ENGINEERING, ENVIRONMENTAL SCIENCE & PLANNING

LYRENACARRIGA WIND FARM

GEOTECHNICAL ASSESSMENT REPORT

Prepared for: MKO



Date: December 2020

Unit 6, Bagenalstown Industrial Park, Royal Oak Road, Bagenalstown, Co. Carlow R21 XA00

T: +353 59 9723800 E: info@ftco.ie

CORK | DUBLIN | CARLOW

www.fehilytimoney.ie

PROJECT NAME: LYRENACARRIGA WIND FARM DOCUMENT NAME: GEOTECHNICAL ASSESSMENT REPORT



LYRENACARRIGA WIND FARM **GEOTECHNICAL ASSESSMENT REPORT MKO**

REVISION CONTROL TABLE, CLIENT, KEYWORDS AND ABSTRACT User is responsible for Checking the Revision Status of This Document

Rev. Nr.	Description of Changes	Prepared by:	Checked by:	Approved by:	Date:
0	Draft for Comment	GK/IH	IH	B. de Hora	28.04.2020
1	Final	GK/IH	IH	B. de Hora	23.07.2020
2	Revised following Client Comments	ін	PJ	B. de Hora	09.11.2020
3	Minor revisions to layout	ін	PJ	B. de Hora	21.12.2020

Client: MKO

Keywords: Inspection, Site visit, Geotechnical

Abstract: Fehily Timoney and Company (FT) was engaged by McCarthy Keville O'Sullivan (MKO) to undertake

a geotechnical assessment of the Lyrenacarriga Wind Farm site.

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1 INTRODUCTION

1.1 Background and Experience

Fehily Timoney and Company (FT), formerly Applied Ground Engineering Consultants Ltd (AGEC) was engaged in August 2018 by McCarthy Keville O'Sullivan to undertake a geotechnical assessment of the proposed Lyrenacarriga wind farm.

FT have been involved in over 100 wind farm developments in both Ireland and the UK at various stages of development i.e. preliminary feasibility, planning, design, construction and operational stage and have established themselves as one of the leading engineering consultancies in stability assessments, geohazard mapping, investigation of peat and non-peat failures and site assessments.

The proposed Lyrenacarriga wind farm site consists of 2 no. clusters of turbines and is located approximately 5km south east of Tallow, Co. Waterford. The wind farm is located east and west of the R634 (reginal road) which links Tallow to the town of Youghal in County Cork.

The proposed development comprises:

- i. Up to 17 no. wind turbines with a tip height of up to 150m and all associated foundations and hardstanding areas,
- ii. 1 no. on-site electrical substation,
- iii. 2 no. temporary construction compounds,
- iv. Upgrading of existing access tracks and provision of new site access tracks required and associated drainage,
- v. Excavation of 3 no. borrow pit areas,
- vi. All associated site development works (including tree felling and permanent met mast).

1.2 Contents of Report

The report includes the following:

- (1) Desk study
- (2) Site reconnaissance
- (3) Summary of ground conditions
- (4) Geotechnical considerations for infrastructural elements
- (5) Management of excavated spoil
- (6) Recommendations



2 SITE DESCRIPTION

The main proposed wind farm is split into 2 no. clusters of turbines which are located within the townlands of Kilcalfmountain and Lyrenacarriga, and adjacent townloands, in Co. Waterford and Co. Cork. The proposed site is predominantly located in Coillte owned forestry, with the remaining areas made up of privately-owned forestry and farmland used for pasture and tillage (Figure 1). The agricultural tillage and pasture lands are predominantly located in the south of the Kilcalfmountain cluster (the western cluster), centrally and within the north of the Lyrenacarriga cluster (the eastern cluster). From a review of the OSI maps the elevation at the proposed site varies approximately between 150 and 200m OD.

The cluster of turbines around the townland of Lyrenacarriga, which borders the R634 (regional road) to the east, has 11 no. proposed turbines labelled T1 to T11. The cluster is typically made up of Coillte forestry to the north and south, with privately owned forestry and agricultural pasture/tillage lands located centrally within the cluster of turbines. The area is generally a mixture of juvenile, mature forestry and deforested areas with some agricultural pasture and tillage land located centrally and to the east of the cluster of turbines. A number of watercourses run through the cluster of turbines which flow into the Glendine River to the south and Blackwater River to the east.

The cluster of turbines around the townland of Kilcalfmountain has 6 no. proposed turbines labelled T12 to T17. From a review of the OSI mapping, the site slopes gently from the north and west towards the south and south east. The topography of the cluster is comprised of generally flat to gently sloping terrain with a mixture of pasture/tillage land, juvenile and mature forestry. The northern portion of the site is drained by a local tributary of the Glenaboy River, while the south portion of the site is drained by a tributary of the Tourig River.



3 DESK STUDY

3.1 Desk Study

The main relevant sources of interest with respect to the site include:

- Geological (soils & bedrock) plans
- Ordnance survey plans
- Literature review of peat and non-peat failures/landslides
- · Previous ground investigation
- Hydrogeology/Groundwater
- · Geological heritage

The Geological Survey of Ireland published mapping (GSI 1995) and on-line database (GSI 2019) was used to verify the soil and bedrock conditions.

The Ordnance Surveys of Ireland (OSI 2019) aerial imagery was reviewed to determine if any notable features or areas of particular interest (from a geotechnical point of view) are present on the site.

The desk study also included a review of both published literature and the GSI online dataset viewer on peat and non-peat failures/landslides in the vicinity of the site.

Other information from the GSI (2019) online database included a review of previous ground investigation information, hydrogeology/groundwater and geological heritage sites.

3.2 Soils, Subsoils & Geology

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A review of the online database (GSI 2019) and published data (GSI 1995) indicates that the site is underlain by the Ballytrasna formation (Figure 3). This formation consists of up to 90% red mudstone with the remaining consisting of pale red fine to medium grained sandstone. Thickness of the formation ranges between 360m to 1500m, with the maximum thickness in the Monavullagh Mountains to the north east in county Waterford. The formation is of Devonian age and is sometimes referred to as "Old Red Sandstone". The member contains significant quartz pebbly sandstone at Ballyvoyle Head and Helvic Head.

The superficial geology consists of predominantly deep well drained mineral soil, with localised areas of shallow well drained mineral soil and alluvial mineral soil (Figure 2).

From the online data there are no fault lines running across the site.

As expected, given the type of rock present, there are no karst features within the site boundary. There are a number of recorded karst features, including an enclosed depression, a cave and a spring, approximately 10km to the north and northeast of the site.

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3.3 **Ordnance Survey Data**

The online ordnance survey imagery (OSI 2019) was reviewed as part of the desk study. Analysis of aerial imagery dating back to 2000 found there has been little change in the proposed wind farm location apart from a localised increase in the forested area with some corresponding localised felling.

A review of the historical mapping for the area identified no notable features from a geotechnical point of view.

Previous Failures 3.4

From a review of the recorded online database (GSI 2019), there are no recorded slope failures at the proposed Lyrenacarriga wind farm site.

The nearest recorded slope failure is located approximately 11km north of the study area. This failure occurred at Lismore Castle, near the Blackwater River in April 2016 and was described as a small landslide on the cliff of the river Blackwater. The cause of the slope failure in this area was recorded as an exceptional rainfall event.

Based on the review carried out, no other slope failures occurred within a 15km radius of the site.

Previous Ground Investigation Data 3.5

Based on a review of the information available (GSI 2019), there are no publicly available intrusive investigation points within a 5km radius of the proposed site boundary.

3.6 Hydrogeology/Groundwater

From a review of the available information (OSI 1997 and GSI 2019), a number of watercourses originate and flow through areas of the proposed wind farm. These watercourse form tributaries of the Glendine River, Glenaboy River and Blackwater River.

A review of the mapping for the area identified no notable features from a geotechnical point of view.

3.7 **Geological Heritage**

Reviewing the available information (GSI 2019), there are no geological heritage features that are within or bordering the proposed wind farm site. There are a number of geological heritage sites a considerable distance from the proposed site.

Approximately 7.5km southeast from the study area, a heritage site is located just outside the town of Youghal. It is not clear what the feature is but is described as road cuts near Youghal Bridge and was identified as part of the National Heritage Plan in 2002.

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In the townland of Bewley, approximately 8.5km northeast from the study area, there are several small caves in low limestone cliffs, alongside the Finisk River. This feature is identified as the Bewley caves.

From the review of the GSI database there are no other geological features within a 10km radius of the proposed site.



4 SITE RECONNAISSANCE

As part of the assessment of potential geotechnical issues at the site, FT carried out a site reconnaissance. This comprised walk-over inspections of the site to record any areas of instability with respect to the proposed wind farm development and to provide a preliminary assessment of the ground conditions.

The following salient geomorphological features were considered:

- Any active, incipient or relict areas of instability
- Presence of shallow valley or drainage lines
- Wet areas
- Any notable change in vegetation
- Inspection of ground conditions
- Slope inclination and break in slope

The survey covered the proposed locations for the turbine bases and associated infrastructure.

The method adopted for carrying out the site reconnaissance relied on practitioners carrying out a visual assessment of the site supplemented with measurement of slope inclinations.

4.1 Findings of Site Reconnaissance

The site reconnaissance comprised a walk-over inspection of the site between 4th and 5th September 2018. Weather conditions for the site visit were dry with sunny spells.

The main findings of the site reconnaissance are as follows:

- (1) The proposed site is predominantly located in Coillte owned forestry, with the remaining areas made up of privately owned forestry and farmland used for pasture and tillage (Appendix A Photos 1 to 3). The proposed wind farm is split into 2 no. clusters of wind turbines which are located in the townlands of Kilcalfmountain and Lyrenacarriga.
- (2) The site typically comprised flat to gently sloping forested areas and agricultural lands, with localised steeply inclined terrain.
- (3) The forested areas have been planted predominantly with conifers with some deciduous plantations.
- (4) Based on the site reconnaissance, no peat was recorded on the site.
- (5) Ground conditions at the site typically comprise topsoil over mineral soil, which was occasionally exposed at the ground surface in the deforested and forested areas. In the pasture/tillage areas, exposures showed dark brown mineral soil.
- (6) From site observations bedrock is estimated to be between 1.5 and 2.0m below ground level. This estimation is based on inspection of a limited number of exposures.



- (7) The mineral soil was typically described as firm to stiff gravelly Clay with occasional cobbles. The upper 1.0 to 1.5m of bedrock was noted as weathered with more intact bedrock present at depth. The bedrock was noted as Sandstone/Mudstone which is consistent with the desk study findings.
- (8) All existing Coillte access tracks on site have been constructed using a founded i.e. excavate & replace technique (Photos 1 & 2). The access tracks for the wind farm will comprise upgrading of existing founded access tracks and construction of new proposed access tracks using excavate and replace construction techniques.
- (9) Slope angles at the turbine locations and other infrastructural elements typically range from 0 to 8 degrees. The slope angle readings are based on site recordings.
- (10) The location of the proposed borrow pits are shown on Figure 1A. The borrow pits will be used to provide suitable granular material during construction of the wind farm infrastructure. Following removal of the rock from a borrow pit, it is proposed to partially restore the borrow pit by storing excavated spoil generated from construction activities.
- (11) No evidence of past failures or any signs of instability were noted on site.
- (12) The conclusions from the site reconnaissance are as follows:
 - (a) The ground conditions recorded on site from a limited number of exposures indicate that typically the site consists of topsoil over mainly cohesive overburden over bedrock.
 - (b) Based on visual inspection of a limited number of exposed ground conditions on site, the bedrock is likely to be suitable for re-use within the lower layers of access roads, crane hardstands, lay down areas, etc.
 - (c) All proposed access tracks for the wind farm will comprise upgrading of existing founded access tracks and construction of new proposed access tracks using excavate and replace construction techniques.
 - (d) No evidence of past failures or any signs of instability were noted on site.

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5 GROUND CONDITIONS

5.1 Soils & Subsoils

Based on the site reconnaissance the superficial deposits are typically topsoil overlying mineral soil overlying bedrock in the forestry areas. While in the agricultural pastures and tillage areas the ground conditions comprised of deeper deposits of mineral soil.

Based on inspections of exposures on site, the mineral soil was typically described as firm to stiff gravelly Clay with occasional cobbles.

A review of the GSI subsoils database indicates that the site is mainly underlain by deep well drained mineral soil with localised shallow mineral soil and alluvial soils. Figure 2 shows the dispersion of soils and subsoils throughout the proposed site.

5.2 Bedrock

The underlying bedrock was described by the Geological Survey of Ireland (GSI 2018) and shown on sheet 22 Geology of East Cork – Waterford (GSI 1995). In the area of the site, one dominant bedrock formation is present.

The site is underlain by the Ballytrasna formation which is described as purple mudstone and sandstone, see Figure 3. There were no rock outcrops recorded across the site. However, from site observations of an existing borrow pit located near to proposed borrow pit no. 3, bedrock is estimated to be between 1.5 and 2.0m below ground level. It should be noted that this estimation is based on inspection of a limited number of exposures.

5.3 Ground Investigation

A ground investigation comprising 27 trial pits was undertaken by HES during May 2020. The trial pit logs are included in Appendix B.

Ground conditions across the site comprise a stiff to very stiff, occasionally soft, slightly gravelly sandy Silt/Clay, typically overlaying a medium dense slightly clayey, sandy Gravel. This gravel deposit may represent a weathered bedrock layer.



6 SUMMARY OF SITE CONDITIONS AT INFRASTRUCTURE LOCATIONS

As part of the site reconnaissance, details of any soft ground and slope angles were recorded throughout the site. Inspections were completed at the proposed turbine locations, access tracks, constructions compounds, substations and borrow pits.

A summary of the site conditions at the proposed infrastructure locations is given in Table 6.1. The slope angles presented in Table 6.1 were recorded on site using a hand-held Silva Clino Master.

Table 6.1: Terrain Type/Land Use & Slope Angle at Proposed Infrastructure Locations

Turbine	Easting	Northing	Terrain/Land Use Type	Slope Angle (°) (1)
T1	603993	587718	Juvenile forestry	3 - 4
T2	603110	587386	Mature forestry	0 - 1
Т3	603576	587412	Agricultural/grassland	2 - 3
Т4	603877	587091	Agricultural/grassland	1 - 2
Т5	603177	586974	Mature forestry	0 - 1
Т6	604338	586514	Agricultural/tillage	2 - 3
Т7	603959	586377	Agricultural/tillage	3 - 4
Т8	603868	585916	Mature forestry	2 - 3
Т9	603487	585581	Mature forestry	1 - 2
T10	603623	585230	Juvenile forestry	7 - 8
T11	603482	586139	Mature forestry	0 - 1
T12	599804	588402	Deforested	0 - 2
T13	599365	588088	Agricultural/grassland	1 - 2
T14	599703	587808	Mature forestry	0 - 1
T15	600078	587585	Agricultural/tillage	1 - 2
T16	599590	587320	Planted forestry	2 - 3
T17	600260	587156	Mature forestry	2 - 3
Substation	604079	586896	Agricultural/grassland	1 - 2
Temporary Construction Compound 1	599170	588066	Felled/planted forestry	0 - 1



Turbine	Easting	Northing	Terrain/Land Use Type	Slope Angle (°) (1)
Temporary Construction Compound 2	ruction 602560 588235		Felled/planted forestry	2 - 3
Borrow Pit 1 599500		588320	Juvenile forestry	3 - 5
Borrow Pit 2	599540	587700	Juvenile forestry	1 - 3
Borrow Pit 3	603520	5855330	Juvenile forestry	3 - 5

Note (1) Slope angle obtained during site survey by AGEC using handheld equipment or taken from contour survey. The slope angle quoted reflects the slope immediately around the infrastructure location.



GEOTECHNICAL CONSIDERATIONS FOR INFRASTRUCTURE ELEMENTS

7.1 **Turbine Foundations**

Based on a review of the GSI information for the area and findings from the site reconnaissance carried out by FT, a preliminary assessment of the likely foundation types found that excavate and replace construction (founded) would be suitable for the turbine foundations.

It should be noted that a confirmatory ground investigation will be carried out at each turbine location prior to construction to confirm the turbine foundation type. The ground investigation will be in the form of a borehole with in-situ SPT testing at 1.0m intervals in the overburden and follow-on rotary core through bedrock.

For gravity type turbine foundations, where the depth of excavation exceeds the minimum required founding depth for the proposed turbine base, up-fill material consisting of granular fill (6N/6P) in accordance with Transport Infrastructure Ireland (TII) requirements shall be used to backfill the excavation to the required founding depth.

Access Tracks 7.2

The guidelines for construction of the access tracks at Lyrenacarriga wind farm are outlined in Section 8.

Up to 10.7km of existing access tracks requiring upgrade are present across the site and based on anecdotal information have been in operation for a significant number of years. The existing access tracks were constructed using the excavate and replace construction technique.

Up to 4.1km of new proposed access roads will be constructed as part of the wind farm construction. Due to the ground conditions the access tracks on site will be founded. The typical make-up of the founded access tracks is a minimum stone thickness of 500mm. The requirement for a layer of geotextile and geogrid and the necessary stone thickness will be confirmed prior to construction.

Crane Hardstands 7.3

The crane hardstands will be constructed using the founded technique (i.e. not floated technique).

Crane hardstands are generally constructed using compacted Class 1/6F material (in accordance with TII requirements) on a suitable sub-formation to achieve the required bearing resistance. The hardstands will be designed for the most critical loading combinations from the crane.

The founding levels for the hardstands may be variable across the site and will be determined prior to construction.

The typical make-up of the hardstands may include up to 1,000mm of granular stone fill with possibly a layer of geotextile and/or geogrid.

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Substation Foundations & Platforms 7.4

The substation platforms will be constructed using the founded technique. The substation foundations may comprise strip/raft foundations under the main footprint of the building with possibly a basement/pit for cable connections.

Substation platforms are generally constructed using compacted Class 1/6F material (in accordance with TII requirements) on a suitable sub-formation to achieve the required bearing resistance.

Given the ground conditions present at the proposed substations, it is envisaged that the foundations will require to be founded on mineral soil or bedrock.

The typical make-up of the substation platform may include up to 750mm of granular stone fill with possibly a layer of geotextile and/or geogrid. At the underside of the substation foundations, a layer of structural up-fill (class 6N/6P - in accordance with TII requirements) will likely be required.

7.5 **Temporary Construction Compound Platforms**

The construction compound platforms will be constructed using founded techniques.

The construction compound platforms are generally constructed using compacted Class 1/6F material (in accordance with TII requirements) on a suitable sub-formation to achieve the required bearing resistance.

The typical make-up of the construction compound platform may include up to 1000mm of granular stone fill with possibly a layer of geotextile and/or geogrid.

7.6 **Borrow Pits**

The guidelines for construction of the 3 no. borrow pits at Lyrenacarriga wind farm are outlined in Section 9.

From the desk study and limited exposures noted during the site reconnaissance, the rock on site is likely to be suitable only for re-use within the lower layers of access roads, crane hardstands, lay down areas, etc.

Imported stone fill is likely to be required to form the upper layers of the infrastructure elements.



8 CONSTRUCTION OF ACCESS TRACKS

Up to 10.7km of existing access tracks requiring upgrade are present across the site and have been in operation for a significant number of years. The existing access tracks were constructed using the excavate and replace construction technique.

Up to 4.1km of new proposed access roads will be constructed as part of the wind farm construction. Due to the ground conditions the access tracks on site will be founded. The typical make-up of the founded access tracks is a minimum stone thickness of 500mm. The requirement for a layer of geotextile and geogrid and the necessary stone thickness will be confirmed prior to construction.

8.1 Upgrade of Existing Access Tracks

This methodology includes procedures that are to be included in the construction to minimise any adverse impact on peat/soil stability. The methodology is not intended to cover all aspects of construction such as drainage and environmental considerations.

- (1) The following guidelines apply:
 - (a) Excavation will be required on one or both sides of the existing access track to a competent stratum.
 - (b) Granular fill to be placed in layers in accordance with the designer's specification.
 - (c) The surface of the existing access track to be overlaid with up to 300mm of selected granular fill.
 - (d) Access roads to be finished with a layer of capping across the full width of the road.
 - (e) A layer of geogrid/geotextile may be required at the surface of the existing access road in areas of excessive rutting (to be confirmed by the site engineer).
 - (f) For excavations in spoil, side slopes shall be not greater than 1 (v): 2. This slope inclination should be reviewed during construction, as appropriate.
- (2) The finished road width will be approximately 5m.
- (3) On side long sloping ground any road widening works required will be done on the upslope side of the existing access road, where possible.
- (4) A final surface layer shall be placed over the existing access track, as per design requirements, to provide a suitable road profile and graded to accommodate wind turbine construction and delivery traffic.

8.2 Construction of New Access Tracks

The excavation of topsoil & spoil and founding of access roads on competent stratum for new access roads will be carried out at various locations on the site. The proposed locations for new excavated access roads on site are shown in Figure 1-A.

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This methodology includes procedures that are to be included in the construction to minimise any adverse impact on peat stability. The methodology is not intended to cover all aspects of construction such as drainage and environmental considerations.

- (1) Interceptor drains will be installed upslope of the access road alignment to divert any surface water away from the construction area.
- (2) Excavation will take place to a competent stratum beneath the topsoil (as agreed with the site designer and resident engineer).
- (3) Road construction will be carried out in sections of approximately 50m lengths i.e. no more than 50m of access road will be excavated without re-placement with stone fill.
- (4) The surface of the excavated access road will be overlaid with up to 500mm of selected granular fill. Granular fill to be placed in layers in accordance with the designer's specification.
- (5) Access roads to be finished with a layer of capping across the full width of the road.
- (6) A layer of geogrid/geotextile may be required at the surface of the competent stratum (to be confirmed by the Site Engineer).
- (7) A final surface layer shall be placed over the excavated road, as per design requirements, to provide a suitable road profile and graded to accommodate wind turbine construction and delivery traffic.



9 MANAGEMENT OF EXCAVATED SPOIL

9.1 Summary of Excavated Spoil & Stone Volumes on Site

A summary of the excavated spoil volumes calculated for the Lyrenacarriga Wind Farm site are given in Table 9.1. These volumes are based on observations recorded by FT (formerly AGEC) during a site walkover.

Table 9.1: Summary of Excavated Spoil Volumes on Site

Infrastructure Element	Typical Dimensions	Spoil (non-peat) Volume (m³)	Comment
17 no. Turbines & Hardstands	turbing foundation with hardstand		Hardstanding area and foundation footprint
Access Roads (includes offsite road upgrade)	,		
Substation	Assumed 21,930m² footprint	26,320	-
Temporary Construction Compound (2) Hardstanding area of 1,925m ²		4,420	Hardstanding areas
Borrow Pit (3) 3 No. borrow pits		90,115	Borrow pit footprint
	Total =	198,980	

Note (1) The location of the infrastructure elements on site are shown on Figure 1A.

Note (2) A factor of 20% (bulking factor of 15% and contingency factor of 5%) has been applied to the excavated spoil volumes to allow for expected increase in volume upon excavation and to allow for a variation in ground conditions across the site.

Note (3) It should be noted that the spoil volumes given in Table 9.1 are indicative and for information purposes only, and subject to detailed design.

Stone volumes required for the construction of access roads, hardstands and turbine bases are summarised in the table below. This stone will be excavated from the proposed borrow pits on the site.



Table 9.2: Summary of Stone Volumes on Site

Infrastructure Element	Typical Dimensions	Stone Volume (m3)	Average Stone Depth (m)	Comment
17 no. Turbines	22m diameter excavation footprint for turbine foundation	4,040	0.5	Foundation footprint
17 no. Hardstands	Hardstand area (varies)	40,910	1.0	Hardstanding area
Access Roads (includes offsite road upgrade)	Assumed 5m running surface with 6m wide development footprint	64,500	Varies	
Substation	Assumed 9,587m² footprint	27,410	1.0	-
Temporary Construction Compound (2)	Hardstanding areas of 3,680m ²	9,200	1.0	Hardstanding areas
		146,060		

Note (1) The location of the infrastructure elements on site are shown on Figure 1

Note (2) A contingency factor of 25% stone volumes to allow for a variation in ground conditions across the site.

9.2 Summary of Spoil Placement/Reinstatement Areas on Site

A summary of the potential spoil placement/reinstatement areas at the Lyrenacarriga Wind Farm site are given in Table 9.3.

Spoil volumes were calculated based on an excavation depth of 3m at turbines, 1m at the substation, 0.5m at the hardstands and 0.5m below the access tracks.

Table 9.3: Summary of Spoil Placement/Reinstatement on Site

Location	Spoil Volume (m³)	Comment
Borrow Pit 1	37,480	
Borrow Pit 2	32,850	
Borrow Pit 3	156,020	
Total =	226,350	



9.3 Guidelines for Borrow Pit Construction and Spoil Placement

Upon removal of the rock from the borrow pits, it is proposed to restore the borrow pits using excavated spoil within cells located inside the borrow pits. The excavated rock from the borrow pits will be used in the construction of the wind farm infrastructure elements (turbine bases, access tracks etc). The contractor excavating the rock will be required to develop the borrow pits in a way which will allow the excavated spoil to be contained safely. It is proposed to construct cells within the borrow pits for the placement of the excavated spoil. This is to allow for the safe placement and grading of the spoil using dumper trucks and excavators. The text below provides design and construction guidelines for the borrow pits.

Figures 4 to 6 show typical construction details for the 3 no. borrow pits.

The borrow pits shall be typically constructed as follows:

- (1) The rock within the proposed borrow pit footprint will be removed by either breaking or blasting, depending on whether it can be excavated, determined from a ground investigation carried out at the proposed borrow pit location. However, it is unlikely that blasting will be required. The ground investigation shall comprise rotary core drilling with associated engineering logging including rock quality designation and strength testing, as required.
- (2) It is proposed to construct the borrow pit so that the base of the borrow pit is below the level of the adjacent section of access road. This may vary and as excavation progresses into the back edge of the borrow pit, the base of the borrow pit may be raised to suit local conditions. Localised deepening of the borrow pit floor may be required depending on extraction operations.
- (3) Depending on the depth and type of rock present in the borrow pits it may be possible to excavate the rock from the borrow pit whilst leaving in place upstands/segments of intact rock which will help to retain the placed spoil. The upstands/segments of intact rock will essentially act as engineered rock buttresses.
- (4) Slopes within the excavated rock formed around the perimeter of the borrow pits will be formed at stable inclinations to suit local in-situ rock conditions. Exposed sections of the rock slopes will be left with irregular faces and declivities to promote re-vegetation and provide a naturalistic appearance.
- (5) The stability of the rock faces within the borrow pit will be inspected by competent personnel upon excavation to ensure stability during construction works and in the long term. This inspection will allow unfavourable rock conditions to be identified and suitable mitigation measures to be applied such as removal of loose rock.
- (6) Where it is not possible to leave upstands/segments of intact rock in place it may be necessary to construct rock buttresses founded on in-situ rock within the borrow pits. The rock buttresses will be constructed of rock fill from the borrow pit excavation. The founding stratum for each rock buttress will be inspected and approved by a competent person.
- (7) Infilling of the spoil will commence at the back edge of the borrow pit and progress towards the borrow pit entrance. The contractor excavating the rock will be required to develop the borrow pits in a way which will allow the excavated spoil to be placed safely.

DOCUMENT NAME: GEOTECHNICAL ASSESSMENT REPORT



- (8) The height of the rock buttresses constructed will be greater than the height of the placed spoil to prevent any surface spoil run-off.
- (9) The use of temporary access ramps and long reach excavators during the placement of the excavated spoil is likely to be required.
- (10) Where possible, the surface of the placed spoil will be shaped to allow efficient run-off of surface water from borrow pit areas.
- (11) An interceptor drain should also be installed upslope of the borrow pit. This drain will divert any surface water away from the borrow pit and hence prevent water from ponding and lodging on the re-instated borrow pit area.
- (12) Control of groundwater within the borrow pits may be required and measures will be determined as part of the confirmatory ground investigation programme. A temporary pump and suitable outfall locations are likely to be required during construction.
- (13) Silting ponds may be required at the lower side/outfall location of the borrow pit.
- (14) Supervision by a geotechnical engineer or appropriately competent person will be carried out for the works.
- (15) All the above general guidelines and requirements will be confirmed by the designer prior to construction. A detailed construction methodology for the borrow pits will be compiled prior to construction.

9.4 **Spoil Placement alongside Excavated Access Tracks**

The following recommendations/best practice guidelines for the placement of spoil alongside the access tracks will be applied during construction. Storage of spoil in this way will be considered an additional measure/storage solution, borrow pit storage is the primary storage solution. Recommendations for placement of excavated material along the access tracks has been included in this report for completeness.

- (1) The potential spoil placement locations to be identified are possibly alongside the existing excavated and proposed new access tracks with cross slopes of less than 10 degrees.
- (2) As a general guide, the spoil placed adjacent to the existing and proposed excavated access tracks will be restricted to a maximum height of 1m over a 5m wide corridor on both sides of the access tracks. It should be noted that the designer will define/confirm the maximum restricted height for the placed spoil.
- The placement of excavated spoil is to be avoided without first establishing the adequacy of the ground (3) to support the load.
- Where there is any doubt as to the stability of the ground then no material shall be placed on to the (4) surface.
- (5) Where practical, it will be ensured that the surface of the placed spoil is shaped to allow efficient runoff of surface water. Shaping of the surface of the spoil should be carried out as placement of spoil within the area progresses. This will reduce the likelihood of debris run-off and ensure stability of the placed spoil.

CLIENT: MKO

PROJECT NAME: LYRENACARRIGA WIND FARM

DOCUMENT NAME: GEOTECHNICAL ASSESSMENT REPORT



- (6) Finished/shaped side slopes in the placed spoil shall be not greater than 1 (v): 2 or 3 (h). This slope inclination will be reviewed during construction, as appropriate.
- (7) Supervision by a geotechnical engineer or appropriately competent person will be carried out for the works.
- (8) An interceptor drain will be installed upslope of the designated spoil placement areas to divert any surface water away from these areas. This will help ensure stability of the placed spoil and reduce the likelihood of debris run-off.
- (9) All the above-mentioned general guidelines and requirements will be confirmed by the designer prior to construction.

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10 SUMMARY & RECOMMENDATIONS

10.1 Summary

FT was engaged by MKO to undertake a geotechnical assessment of the main proposed wind farm site. The assessment comprised a site walkover, desk study, summary of ground conditions, geotechnical considerations for infrastructure and an assessment of spoil and stone volumes for the proposed development.

The main findings of the site reconnaissance are as follows:

- (a) The ground conditions recorded on site from a limited number of exposures indicate that typically the site consists of topsoil over mineral soil over bedrock.
- (b) Based on visual inspection of a limited number of exposed ground conditions on site, the bedrock is likely to be suitable for re-use within the lower layers of access roads, crane hardstands, lay down areas, etc.
- (c) All proposed access tracks for the wind farm will comprise upgrading of existing founded access tracks and construction of new proposed access tracks using excavate and replace construction techniques.
- (d) No evidence of past failures or any signs of instability were noted on site.

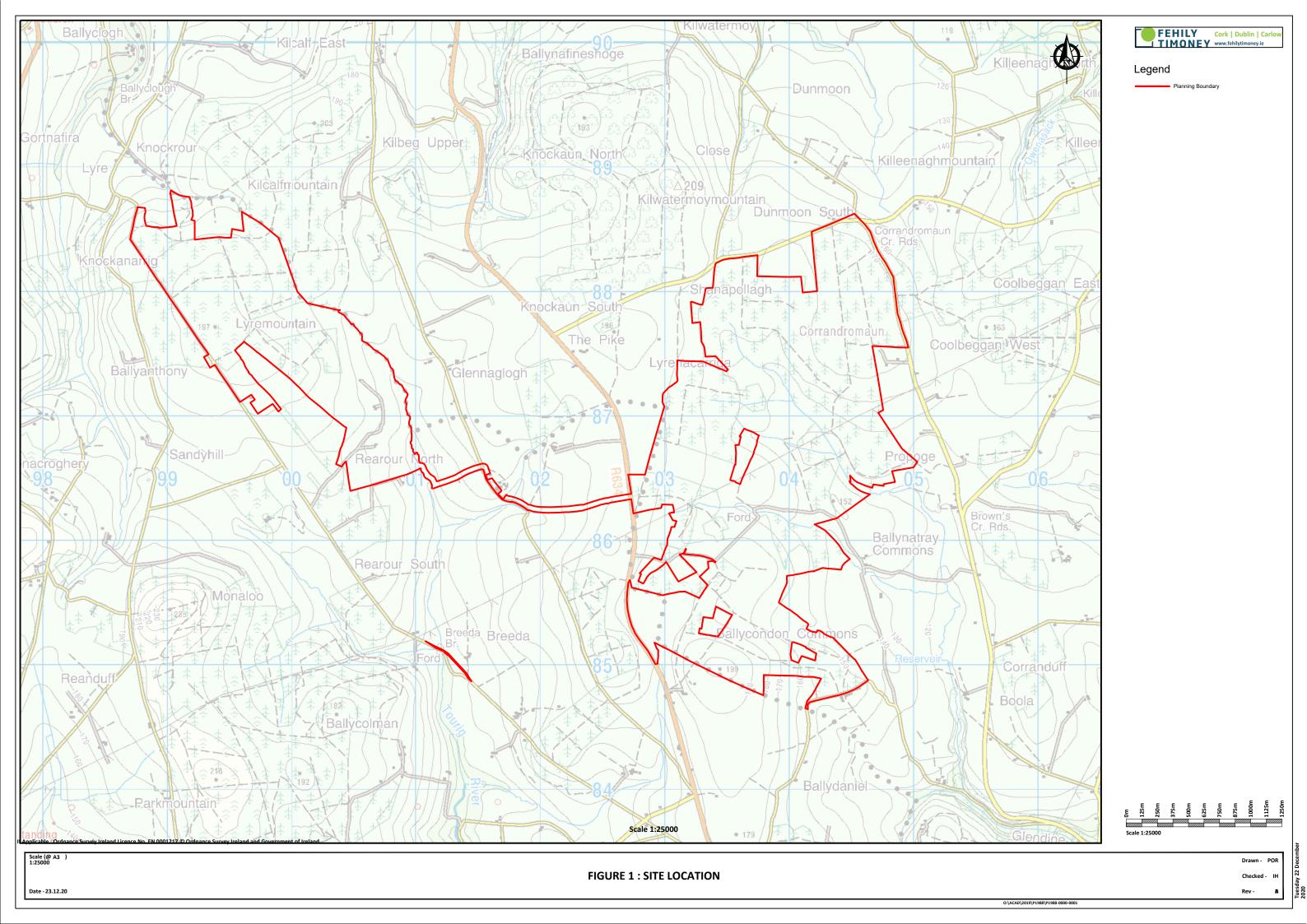
A network of existing tracks is present on the site. It is proposed to upgrade these existing tracks and construct additional tracks to provide access to the turbine locations.

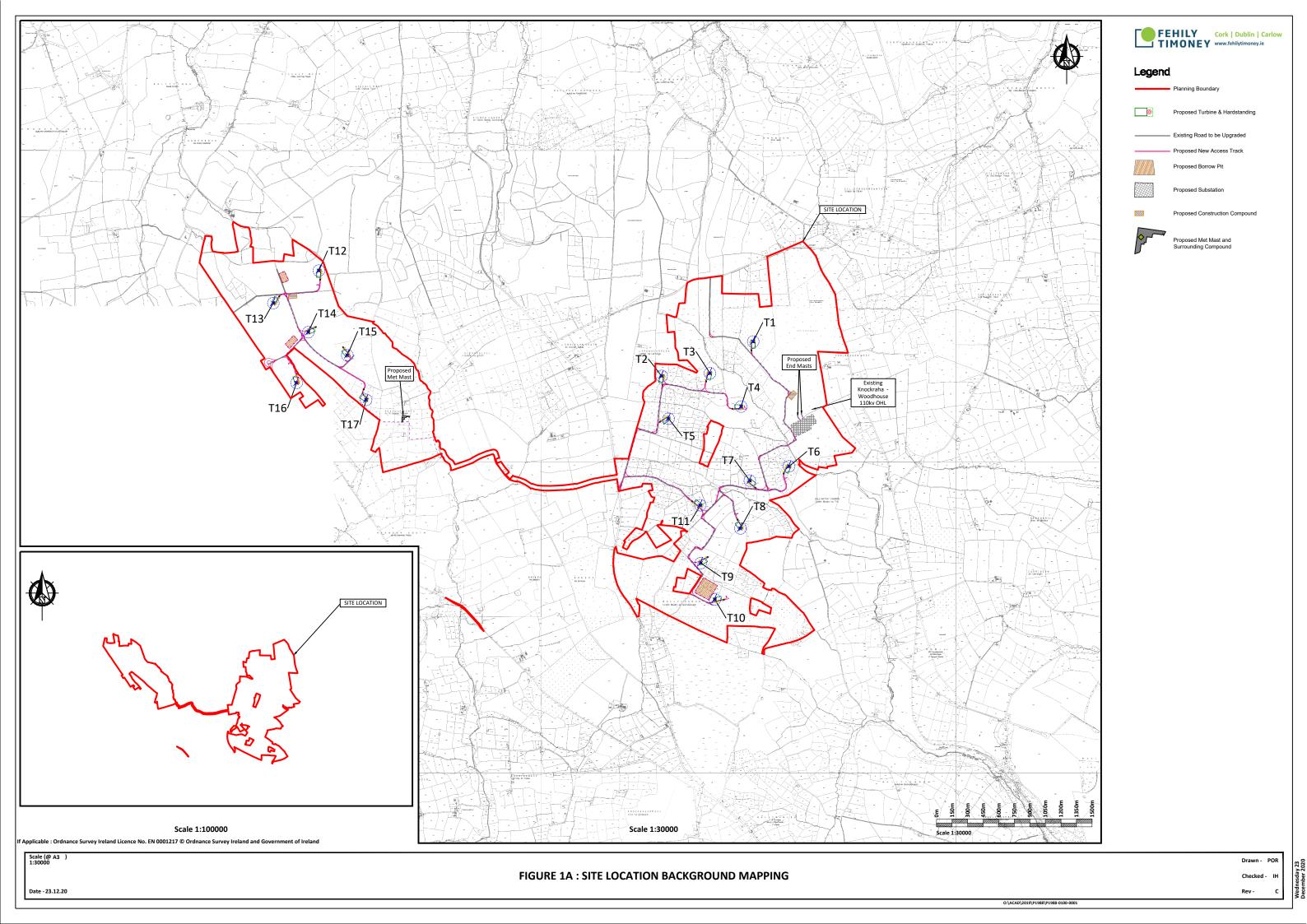
A total of three borrow pits are proposed for the site. The borrow pits are proposed to provide sufficient stone for the proposed development and also to provide a suitable storage area for spoil material generated from construction activities. Two of the borrow pits are located in the western cluster of the site, and one is in the eastern cluster.

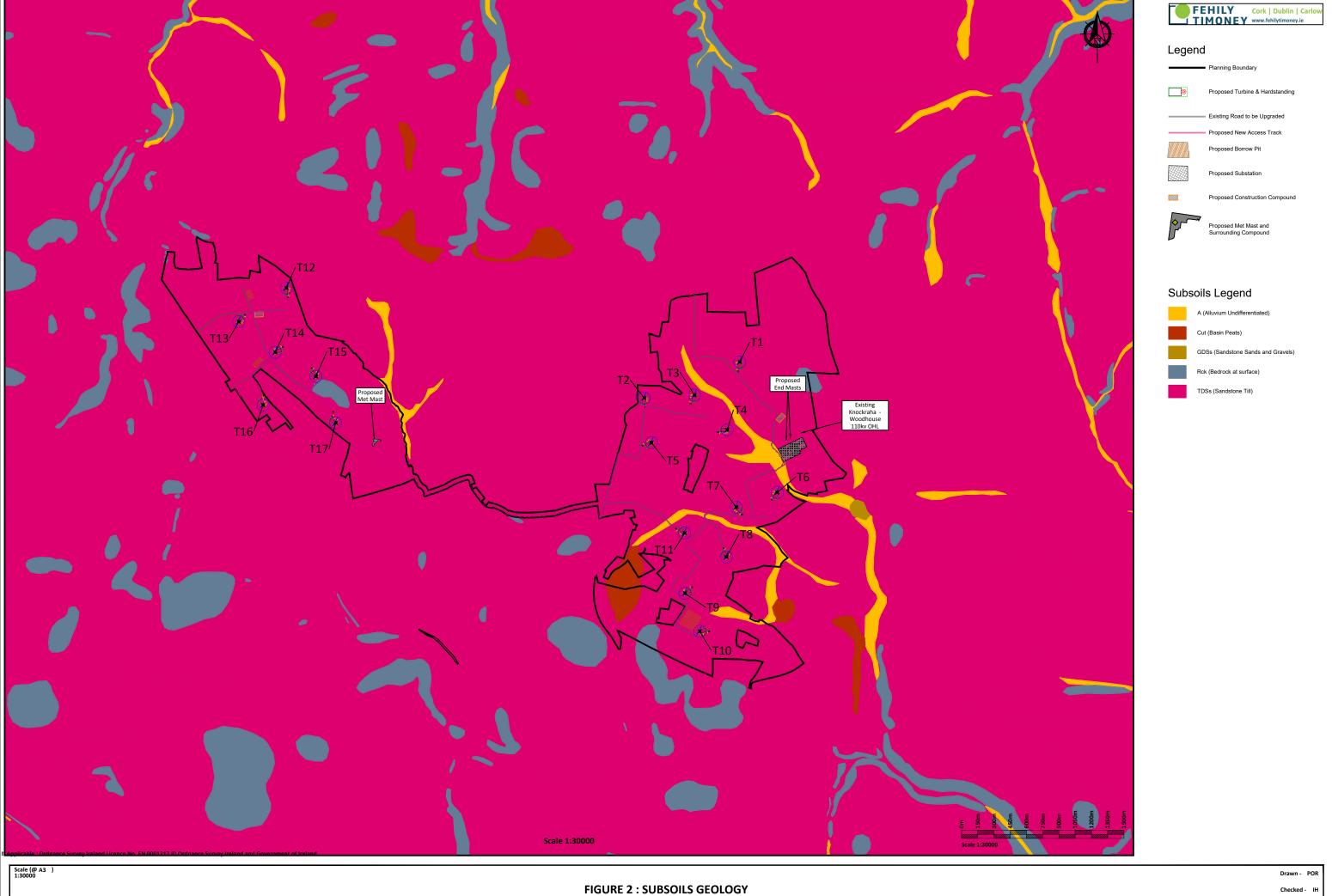
10.2 Recommendations

The following general recommendations are given.

- (1) Given the absence of peat on the site, all infrastructure will be founded.
- (2) Material excavated from access tracks and other infrastructure locations will be stored in the three borrow pits proposed for the site. If required, additional storage can be obtained by placing material alongside access tracks, as described in Section 9.4.



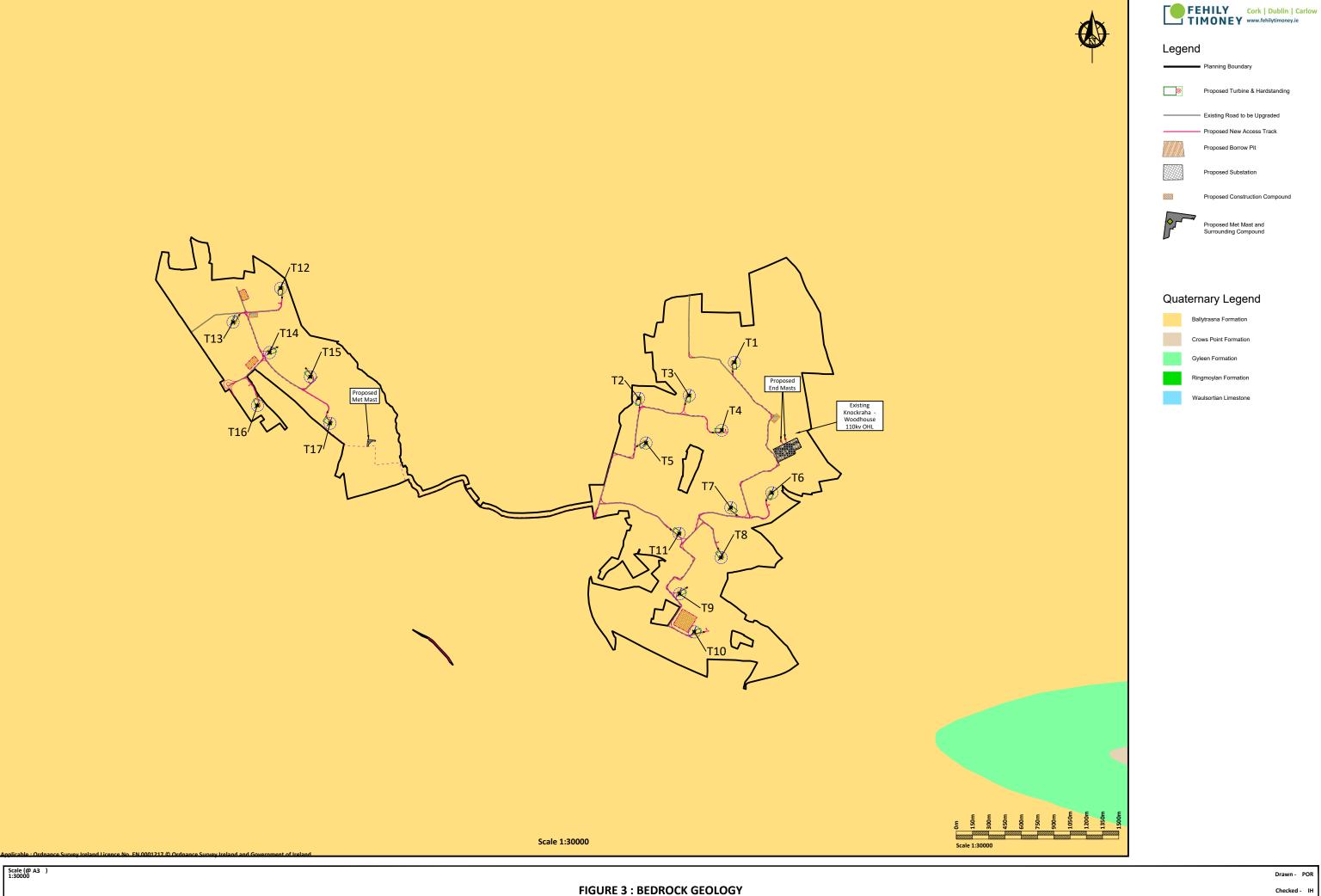




Date - 23.12.20

Checked - IH

Rev -



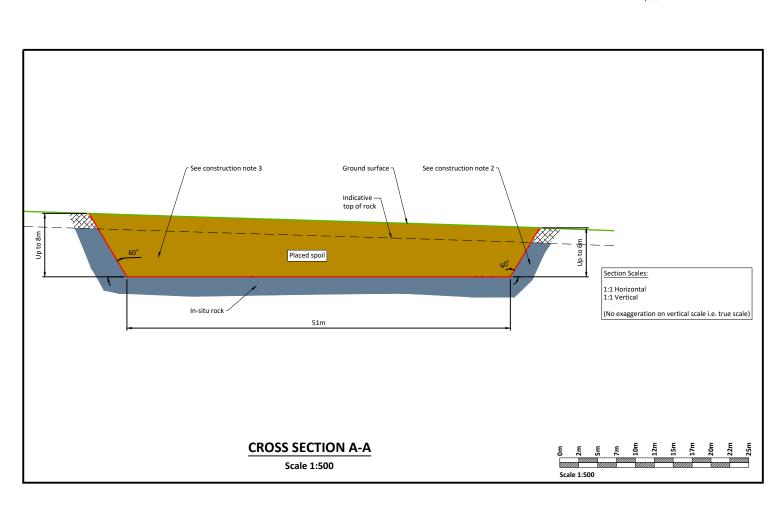
Drawn - POR Checked - IH

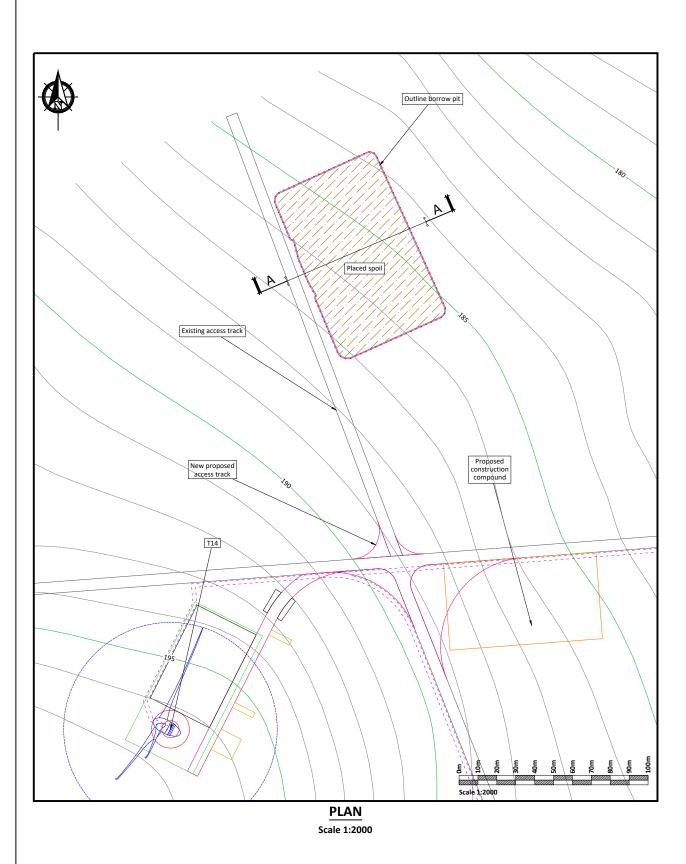
Rev -



Construction Notes Borrow Pit:

- (1) It is proposed to construct the borrow pit so that the base of the borrow pit is below the level of the adjacent section of access road. Depending on the type and condition of rock present in the borrow pit it may be possible to excavate the rock from the borrow pit whilst leaving in place upstands/segments of intact rock which will help to retain the placed spoil. The upstands/segments of intact rock will essentially act as engineered rock buttresses within the borrow pit.
- (2) Slopes within the excavated rock formed around the perimeter of the borrow pit should be formed at stable inclinations to suit local in-situ rock conditions.
- (3) Infilling of should commence at the back edge of the borrow pit and progress towards the borrow pit entrance. Excavation and infilling of the borrow pit will need to be sequenced and programmed. Leaving in place upstands/segments of intact rock which will help to retain the placed spoil and will allow the borrow pit to be developed and infilled in cells.
- (4) The contractor excavating the rock will be required to develop the borrow pit in a way which will allow the excavated spoil to be reinstated safely.
- (5) Where possible, the surface of the placed spoil should be shaped to allow efficient run-off of surface water from the placed arising's.
- (6) Control of groundwater within the borrow pit may be required and measures will be determined as part of the ground investigation programme.
- (7) All the above-mentioned general guidelines and requirements should be confirmed by the designer prior to construction.
- (8) Further guidelines on the construction of the borrow pit is included within Section 9.1 of the Geotechnical Assessment Report.





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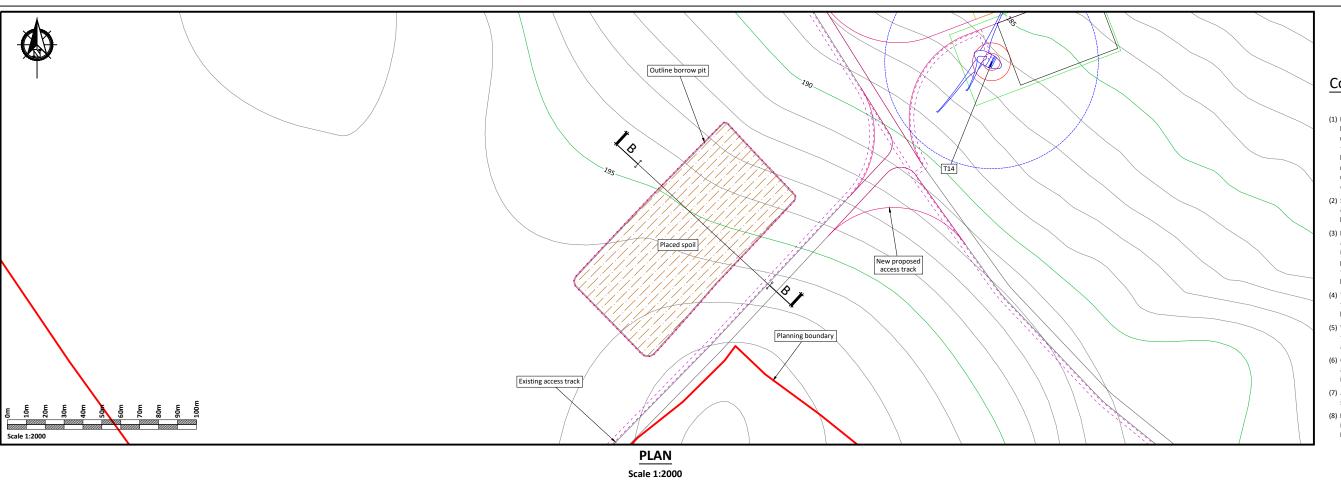
Scale (@ A3) 1:2000

Date - 23.12.20

FIGURE 4 : BORROW PIT - PLAN & CROSS SECTION DETAILS

Checked - IH

Drawn - POR





Construction Notes Borrow Pit:

- (1) It is proposed to construct the borrow pit so that the base of the borrow pit is below the level of the adjacent section of access road. Depending on the type and condition of rock present in the borrow pit it may be possible to excavate the rock from the borrow pit whilst leaving in place upstands/segments of intact rock which will help to retain the placed spoil. The upstands/segments of intact rock will essentially act as engineered rock buttresses within the borrow pit.
- (2) Slopes within the excavated rock formed around the perimeter of the borrow pit should be formed at stable inclinations to suit local in-situ rock conditions.
- (3) Infilling of should commence at the back edge of the borrow pit and progress towards the borrow pit entrance. Excavation and infilling of the borrow pit will need to be sequenced and programmed. Leaving in place upstands/segments of intact rock which will help to retain the placed spoil and will allow the borrow pit to be developed and infilled in cells.
- (4) The contractor excavating the rock will be required to develop the borrow pit in a way which will allow the excavated spoil to be reinstated safely.
- (5) Where possible, the surface of the placed spoil should be shaped to allow efficient run-off of surface water from the placed arising's.
- (6) Control of groundwater within the borrow pit may be required and measures will be determined as part of the ground investigation programme.
- (7) All the above-mentioned general guidelines and requirements should be confirmed by the designer prior to construction.
- (8) Further guidelines on the construction of the borrow pit is included within Section 9.1 of the Geotechnical Assessment Report.

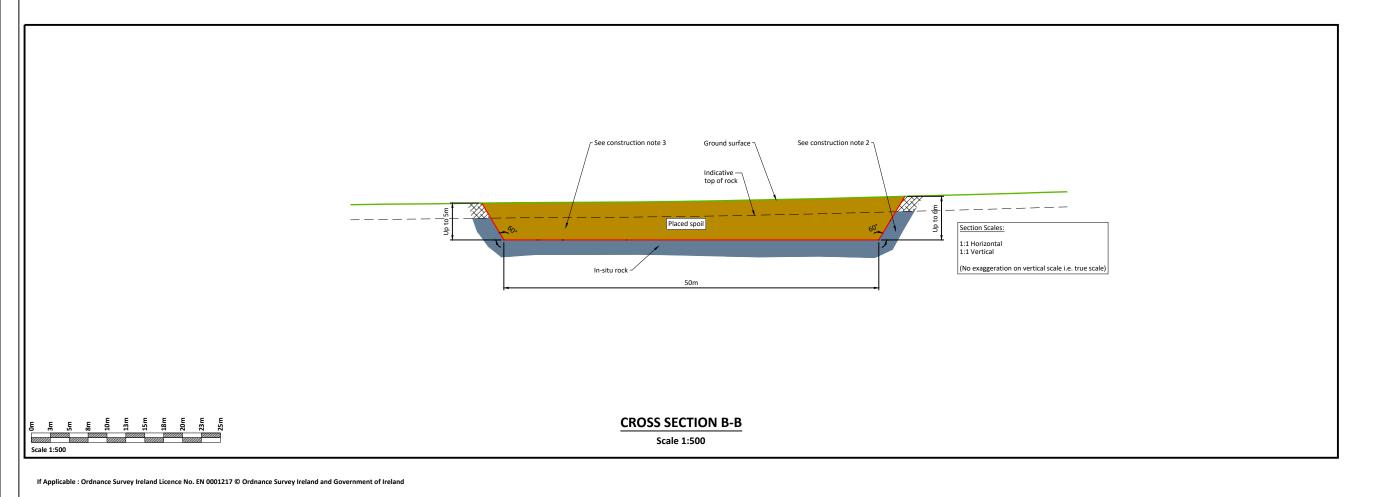


FIGURE 5: BORROW PIT - PLAN & CROSS SECTION DETAILS

Checke Rev -

Drawn - POR

Checked - IH

Scale (@ A3) 1:2000



Construction Notes Borrow Pit:

- (1) It is proposed to construct the borrow pit so that the base of the borrow pit is below the level of the adjacent section of access road. Depending on the type and condition of rock present in the borrow pit it may be possible to excavate the rock from the borrow pit whilst leaving in place upstands/segments of intact rock which will help to retain the placed spoil. The upstands/segments of intact rock will essentially act as engineered rock buttresses within the borrow pit.
- (2) Slopes within the excavated rock formed around the perimeter of the borrow pit should be formed at stable inclinations to suit local in-situ rock conditions.
- (3) Infilling of should commence at the back edge of the borrow pit and progress towards the borrow pit entrance. Excavation and infilling of the borrow pit will need to be sequenced and programmed. Leaving in place upstands/segments of intact rock which will help to retain the placed spoil and will allow the borrow pit to be developed and infilled in cells.
- (4) The contractor excavating the rock will be required to develop the borrow pit in a way which will allow the excavated spoil to be reinstated safely.
- (5) Where possible, the surface of the placed spoil should be shaped to allow efficient run-off of surface water from the placed arising's.
- (6) Control of groundwater within the borrow pit may be required and measures will be determined as part of the ground investigation programme.
- (7) All the above-mentioned general guidelines and requirements should be confirmed by the designer prior to construction.
- (8) Further guidelines on the construction of the borrow pit is included within Section 9.1 of the Geotechnical Assessment

See construction note 3 See construction note 2 -Ground surface top of rock Section Scales: (No exaggeration on vertical scale i.e. true scale) **CROSS SECTION C-C** Scale 1:500

FIGURE 6: BORROW PIT - PLAN & CROSS SECTION DETAILS

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Scale (@ A3) 1:2000

Date - 23.12.20

Drawn - POR Checked - I

Rev -



CONSULTANTS IN ENGINEERING & ENVIRONMENTAL SCIENCES

APPENDIX A

Site Inspection Photographs



Photo 1: Example of an existing access track on site



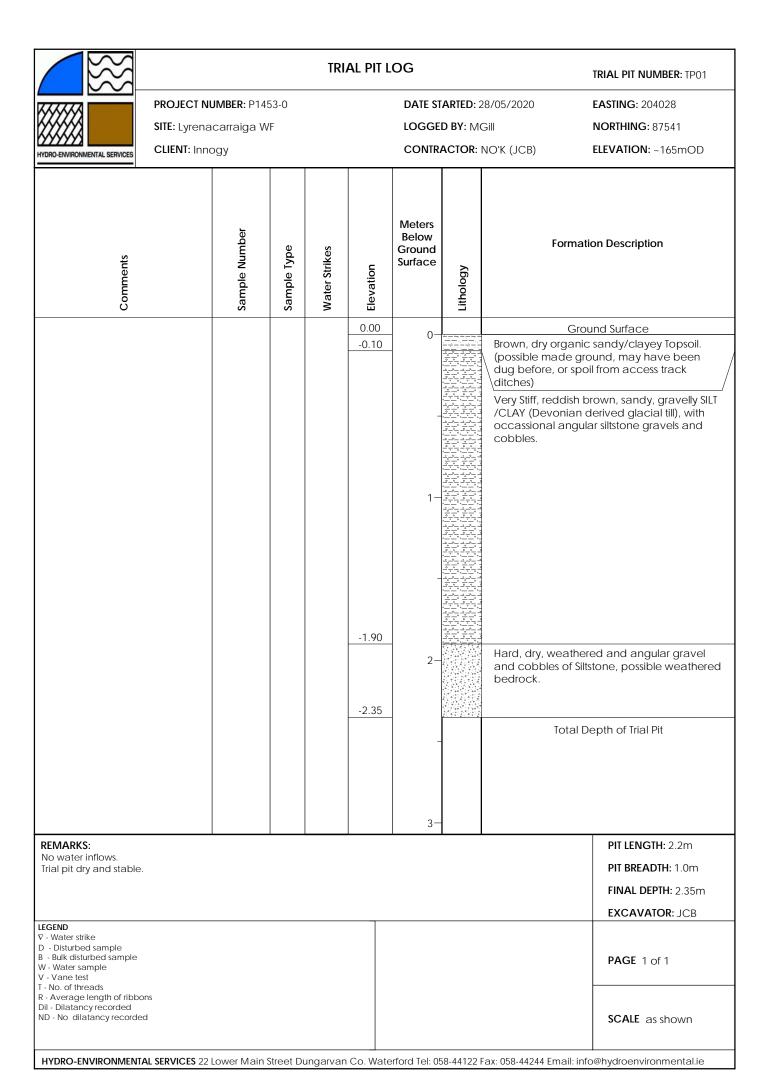
Photo 2: Example of an existing access track on site



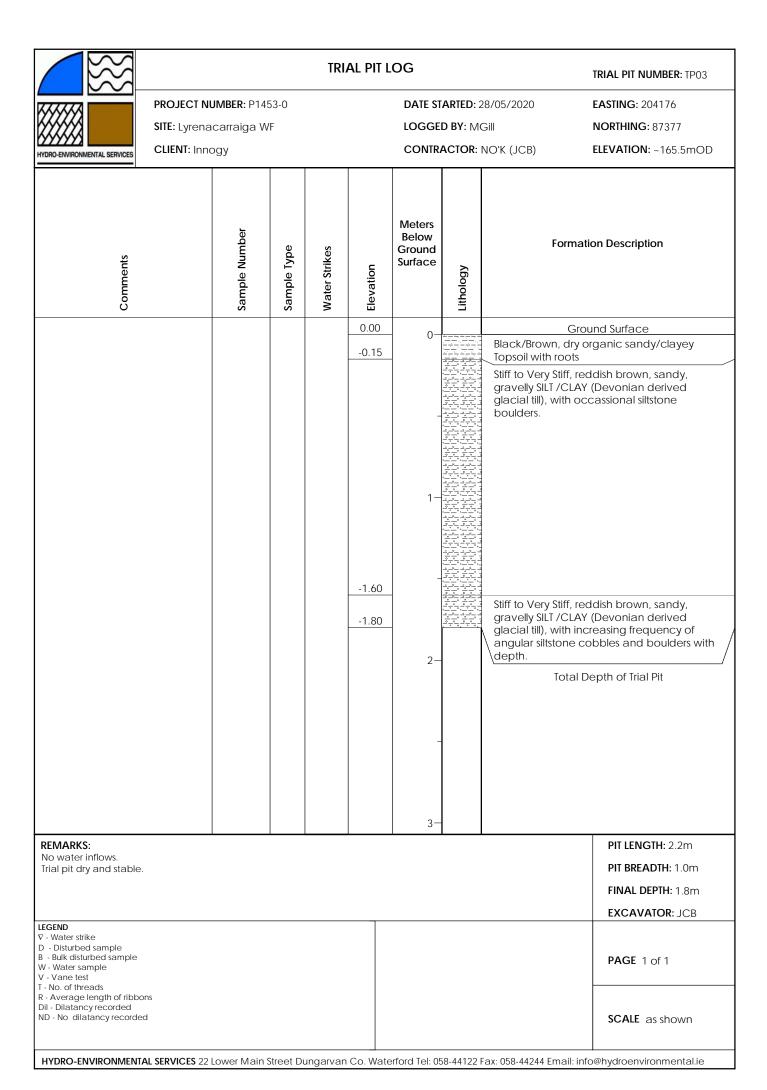
CONSULTANTS IN ENGINEERING, ENVIRONMENTAL SCIENCE & PLANNING

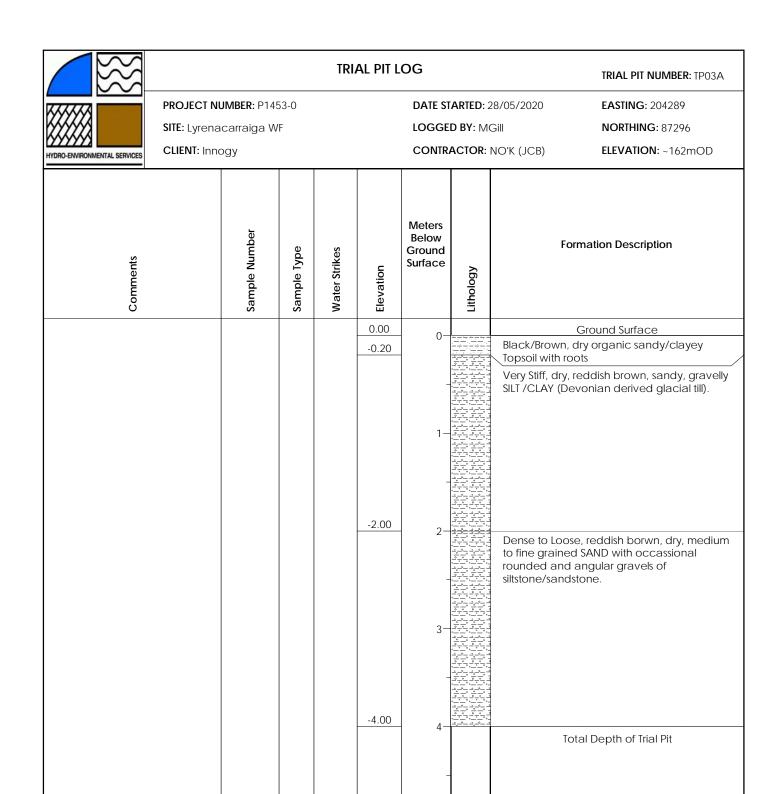
APPENDIX B

Ground Investigation
Information



				TRI	AL PIT L	.OG			TRIAL PIT NUMBER: TP02	
	PROJECT NUMBER: P1453-0 DATE STARTI					ARTED: 2	28/05/2020	EASTING : 204099		
		SITE: Lyrenacarraiga WF				LOGGE	D BY: MO	Gill	NORTHING: 87461	
	CLIENT: Inno	ogy				CONTR	ACTOR:	NO'K (JCB)	ELEVATION: ~165mOD	
Comments		Sample Number	Sample Type	Water Strikes	Elevation	Meters Below Ground Surface	Lithology	Forma	ation Description	
					0.00	0-			ound Surface	
					-0.20					
					-2.70	1— 2—		Brown, dry organic sandy/clayey Topsoil with grass rootlets Stiff to Very Stiff, reddish brown, sandy, gravelly SILT /CLAY (Devonian derived glacial till), with occassional angular siltstone cobbles/boulders at depth.		
								Total	Depth of Trial Pit	
						3-				
REMARKS: No water inflows.									PIT LENGTH: 2.2m	
Trial pit dry and stabl	e.								PIT BREADTH: 1.0m	
								FINAL DEPTH: 2.7m		
LEGEND									EXCAVATOR: JCB	
LEGEND V - Water strike D - Disturbed sample B - Bulk disturbed sample W - Water sample V - Vane test							PAGE 1 of 1			
R - Average length of ribl Dil - Dilatancy recorded	- No. of threads - Average length of ribbons Dil - Dilatancy recorded ND - No dilatancy recorded								SCALE as shown	
HYDRO-ENVIRONMEN	ITAL SERVICES 22	Lower Main S	Street Du	ıngarvan	Co. Wate	erford Tel: 0	58-44122 F	ax: 058-44244 Email: ir	nfo@hydroenvironmental.ie	





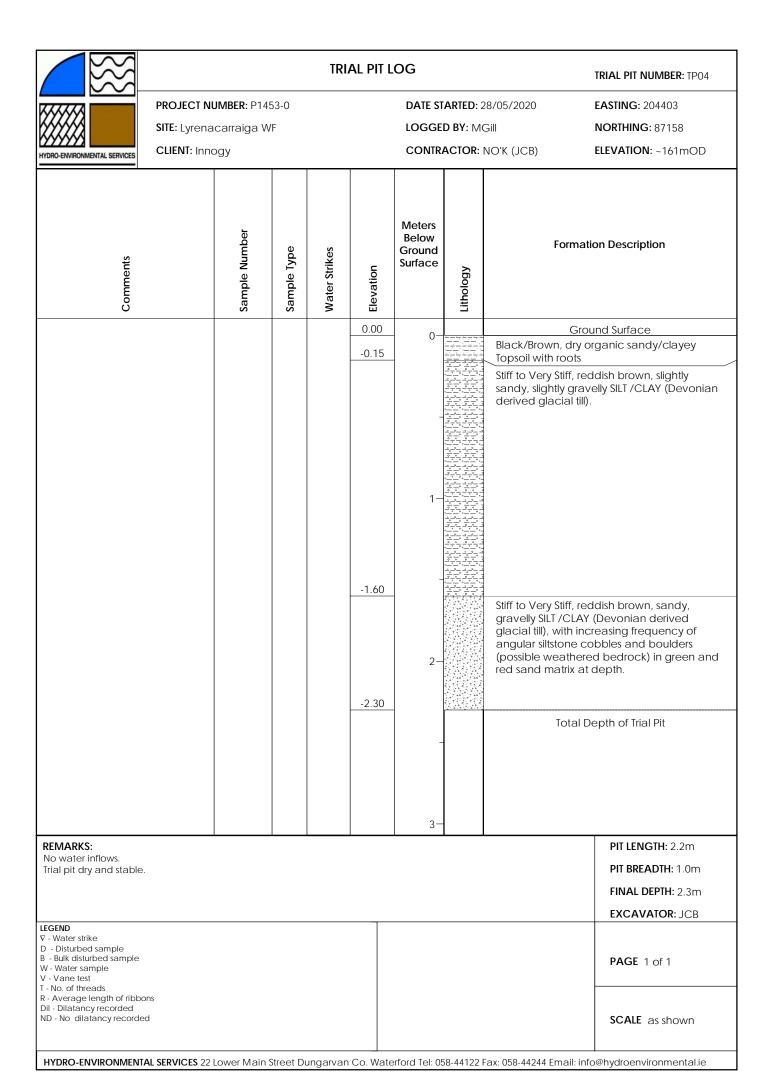
Face of old borrow pit cleaned and logged. PIT BREADTH: n/a No water seepages noted. Face of pit was dry and stable. FINAL DEPTH: n/a **EXCAVATOR: JCB** LEGEND ∇ - Water strike D - Disturbed sample - Bulk disturbed sample PAGE 1 of 1 W - Water sample V - Vane test T - No. of threads R - Average length of ribbons Dil - Dilatancy recorded ND - No dilatancy recorded **SCALE** as shown

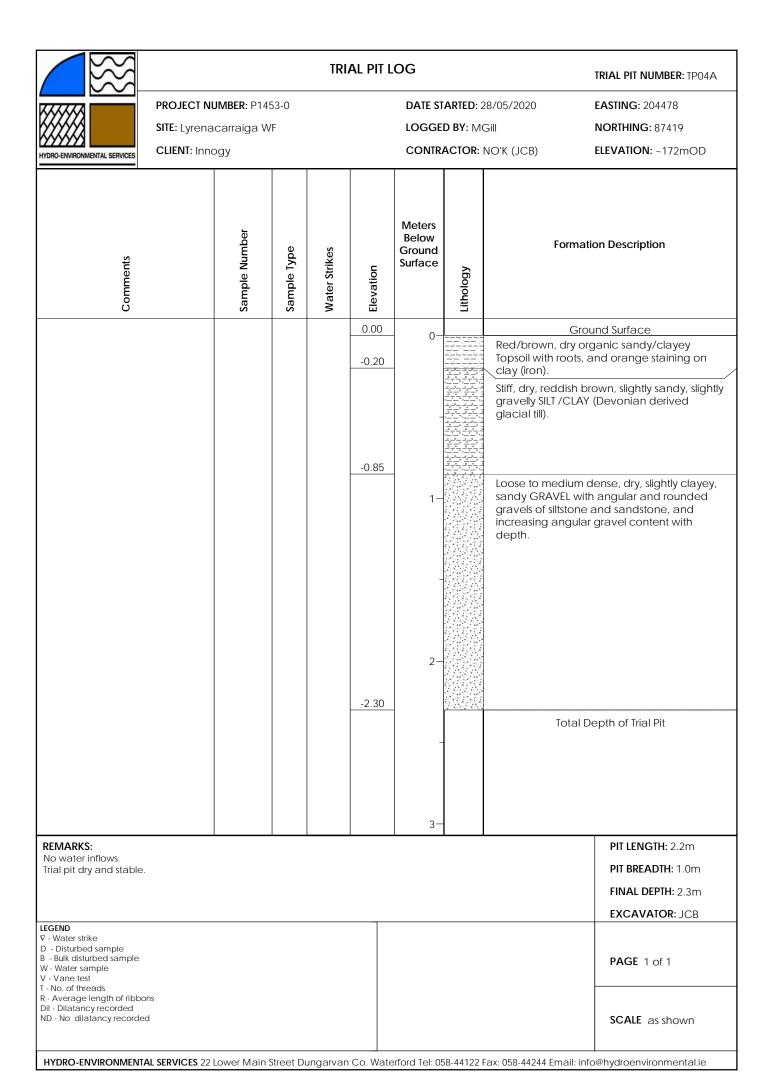
HYDRO-ENVIRONMENTAL SERVICES 22 Lower Main Street Dungarvan Co. Waterford Tel: 058-44122 Fax: 058-44244 Email: info@hydroenvironmental.ie

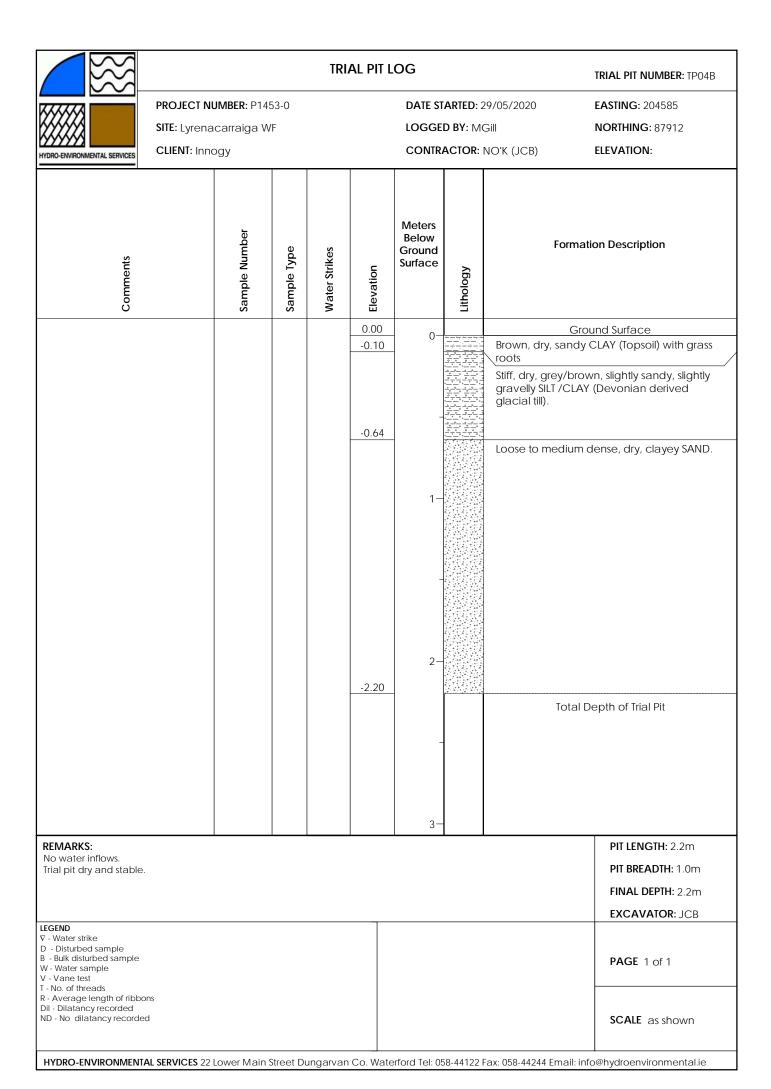
REMARKS:

5

PIT LENGTH: n/a









TRIAL PIT NUMBER: TP06

PROJECT NUMBER: P1453-0 **DATE STARTED**: 29/05/2020

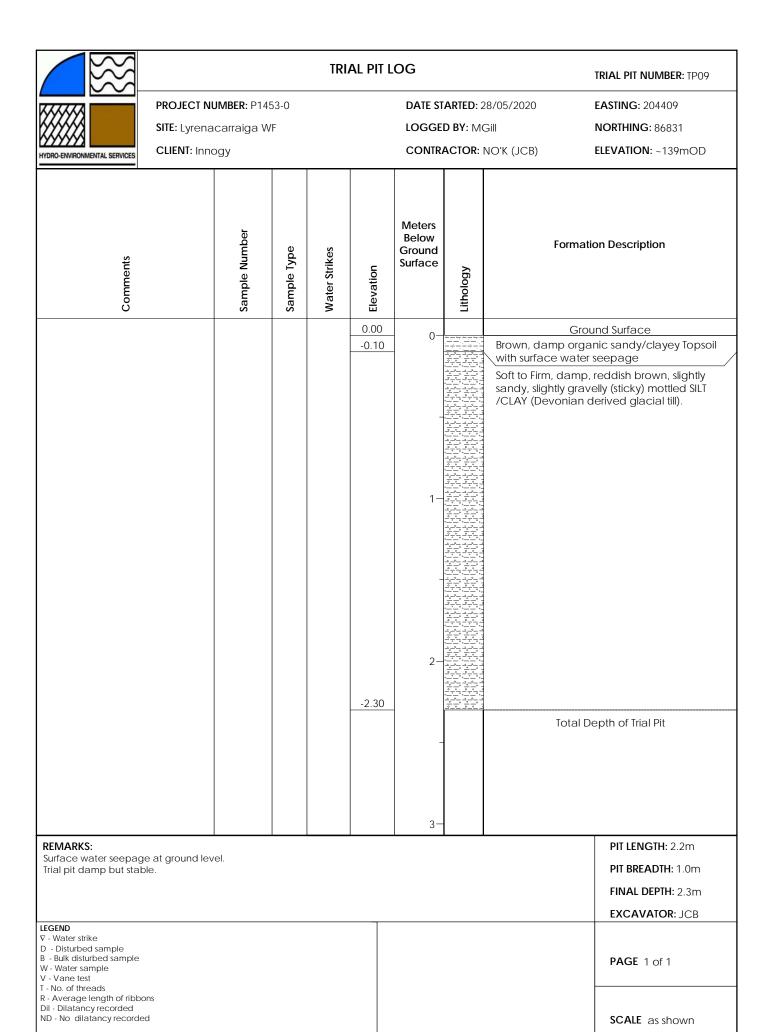
EASTING: 204143

SITE: Lyrenacarraiga WF

NORTHING: 86979

LOGGED BY: MGill

DRO-ENVIRONMENTAL SERVICES CLIENT: Inn	ogy				CONTRA	ACTOR: I	NO'K (JCB) ELEV	ATION: ~139.5mOD
Comments	Sample Number	Sample Type	Water Strikes	Elevation	Meters Below Ground Surface	Lithology	Formation De	escription
				0.00	0-		Ground S	urface
							Dark brown, organic, sto (ploughed field, close to	oney clayey Topsoil stream).
				-0.50	_		Medium dense, red/bro	wn. gravelly CLAY
				-0.70			(gravelly clay layer abov	ve main till).
					1-		Very Stiff, damp, reddish slightly sandy, slightly gra (Devonian derived glac occassional cobbles of	velly SILT /CLAY al till), with
					-			
				-1.80				
strong water inflow			¥	1100	2-		Wet, Loose to Medium d brown, sandy, angular a GRAVEL of siltstone/sand	ind sub-rounded
				-2.30				
					-		Total Depth	of Trial Pit
EMARKS:					3-			T LENGTH: 2.2m
EMARKS: Vater inflow @ 2.1mbgl. rial pit collapsing below 1.8mbgl afte	er water inflo	Λ/						T BREADTH: 1.0m
nai pit collapsing below 1.0mbgl att	J. Water IIIIOV	, v .						NAL DEPTH: 2.3m
								(CAVATOR: JCB
GEND - Water strike - Disturbed sample - Bulk disturbed sample - Water sample - Vane test No. of threads							P	AGE 1 of 1
- Average length of ribbons I - Dilatancy recorded D - No dilatancy recorded								CALE as shown





DATE STARTED: 28/05/2020

EASTING: 204459

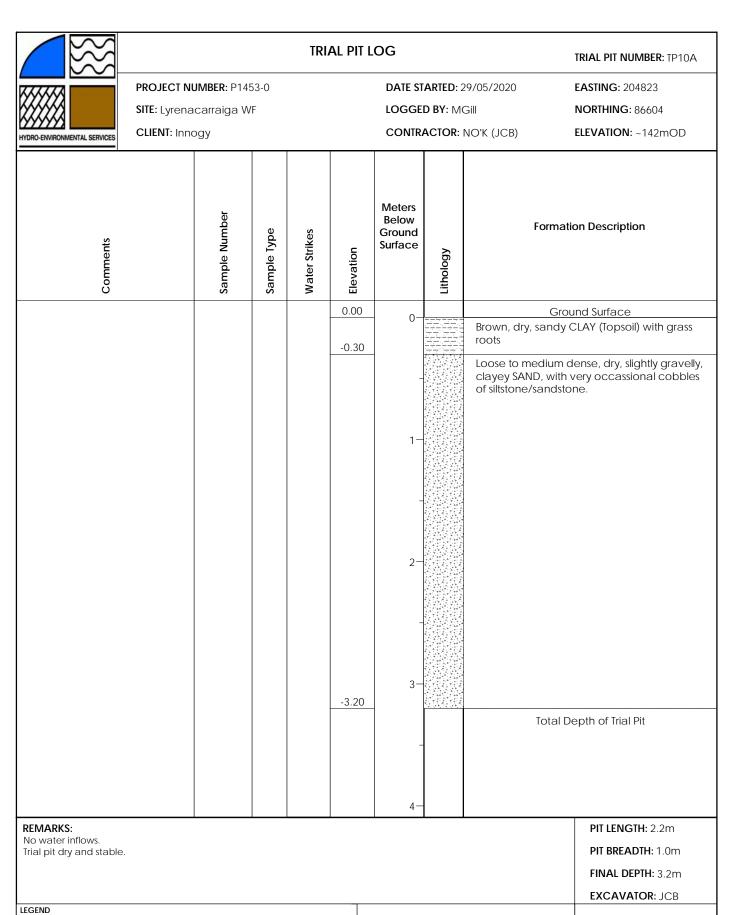
TRIAL PIT NUMBER: TP10

SITE: Lyrenacarraiga WF

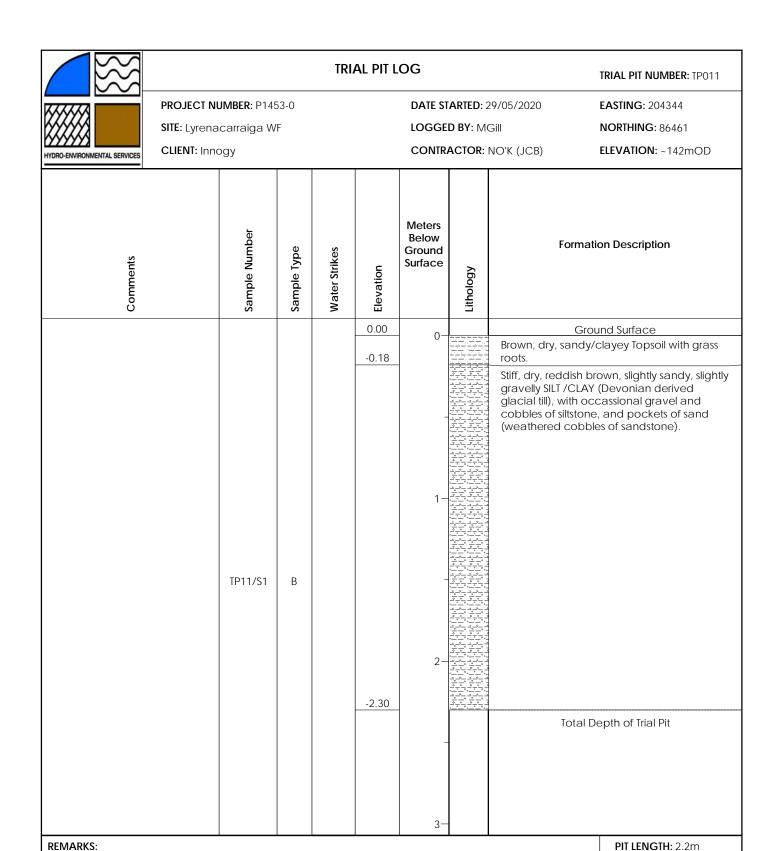
PROJECT NUMBER: P1453-0

LOGGED BY: MGill NORTHING: 86731

YDRO-ENVIRONMENTAL SERVICES	CLIENT: Inno					CONTRACTOR: NO'K (JCB) ELEVATION: ~136m				
Comments		Sample Number	Sample Type	Water Strikes	Elevation	Meters Below Ground Surface	Lithology	Formatio	on Description	
			0.00 Rade Sami	0.00	1-		Ground Surface Track side drain/ditch, with red/brown, damp organic sandy/clayey Topsoil with surface water seepage. Soft to Firm, damp, reddish brown, slightly sandy, slightly gravelly (sticky) mottled SILT /CLAY (Devonian derived glacial till). (Excavated material clumps together when excavated)			
REMARKS: Surface water seepa Trial pit damp from 1	ge at ground lev 0-2.6mbgl but sta	vel. able.				3-		TOTAL DE	PIT LENGTH: 2.2m PIT BREADTH: 1.0m	
EGEND - Water strike - Disturbed sample - Bulk disturbed sample / Water sample - Vane test - No. of threads - Average length of ribt iil - Dilatancy recorded D - No dilatancy recorded	oons								FINAL DEPTH: 2.6m EXCAVATOR: JCB PAGE 1 of 1 SCALE as shown	



V - Water strike D - Disturbed sample B - Bulk disturbed sample W - Water sample V - Vane test T - No. of threads R - Average length of ribbons Dil - Dilatancy recorded ND - No dilatancy recorded HYDRO-ENVIRONMENTAL SERVICES 22 Lower Main Street Dungarvan Co. Waterford Tel: 058-44122 Fax: 058-44244 Email: info@hydroenvironmental.ie

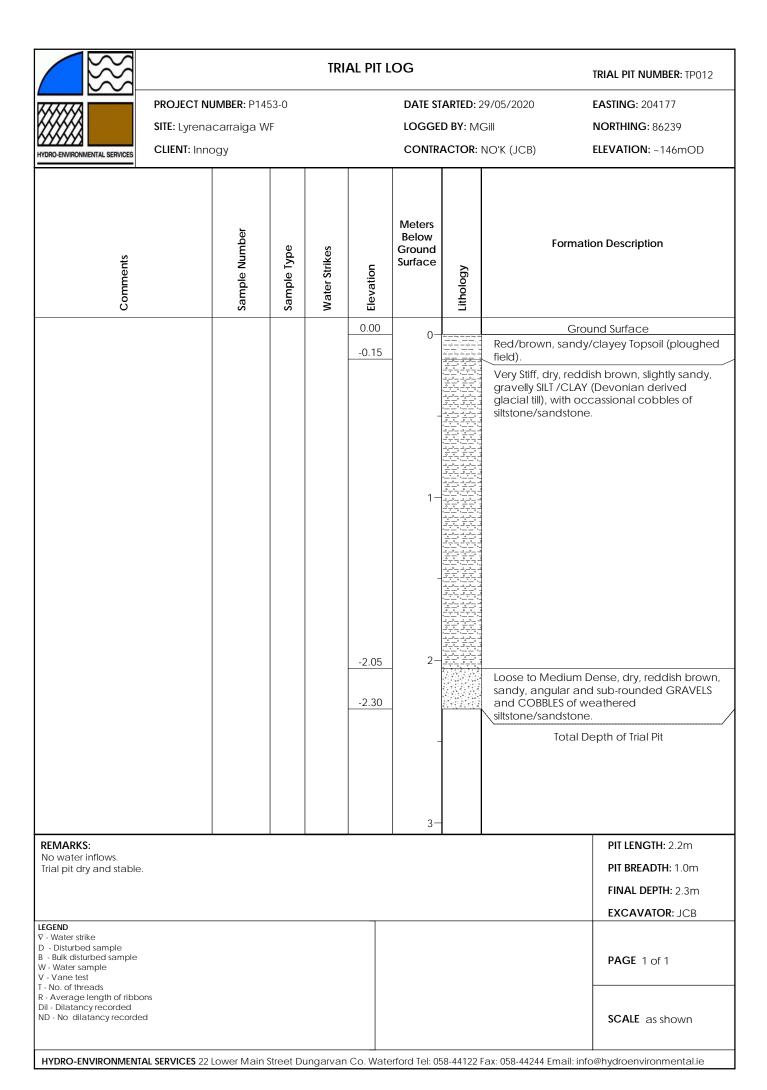


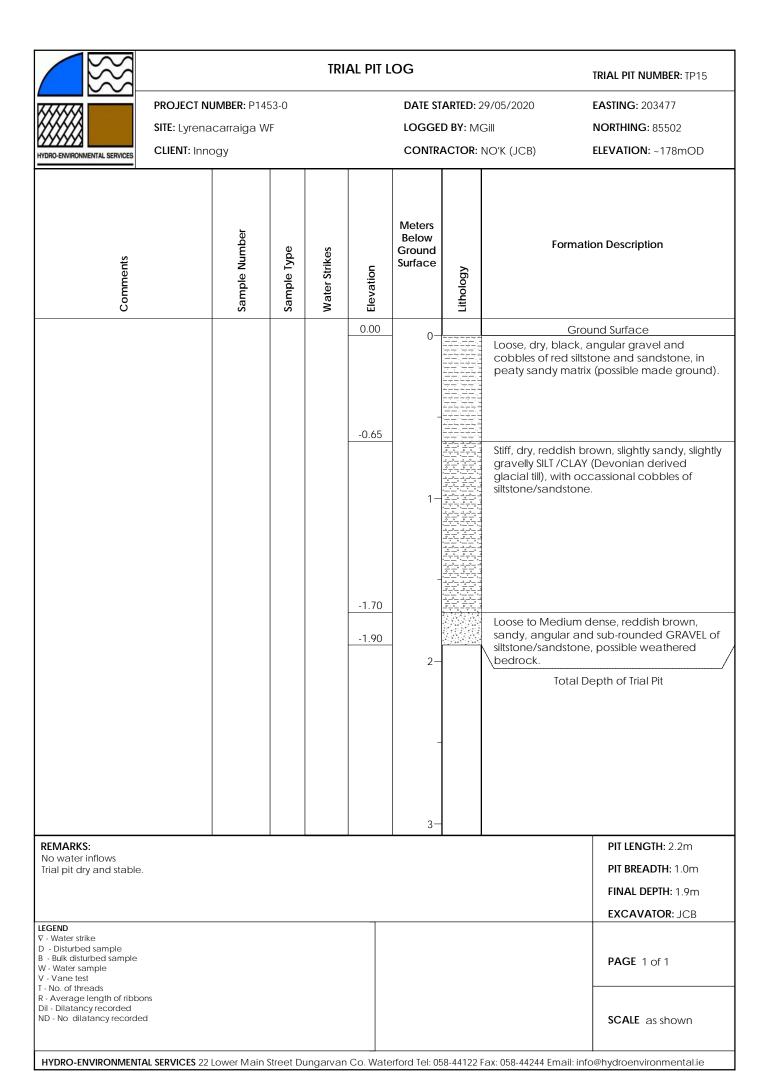
	FINAL DEPTH: 2.3m
	EXCAVATOR: JCB
LEGEND ∇ - Water strike D - Disturbed sample B - Bulk disturbed sample W - Water sample V - Vane test T - No. of threads R - Average length of ribbons Dil - Dilatancy recorded ND - No dilatancy recorded	PAGE 1 of 1 SCALE as shown

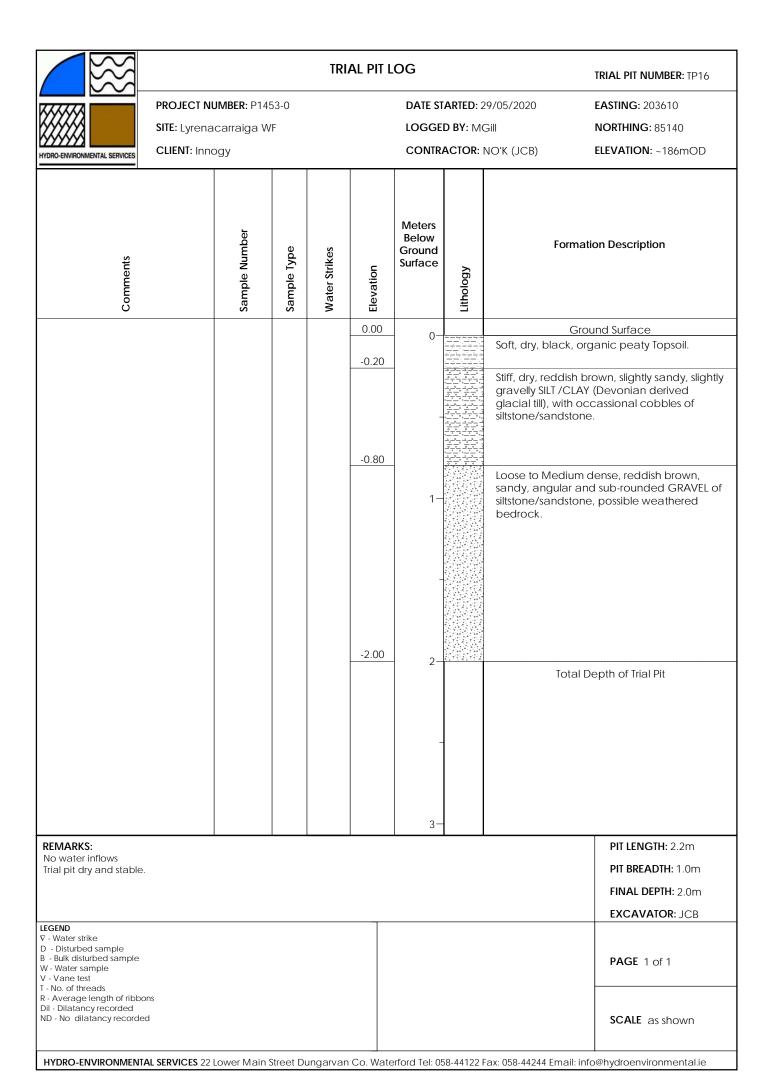
PIT BREADTH: 1.0m

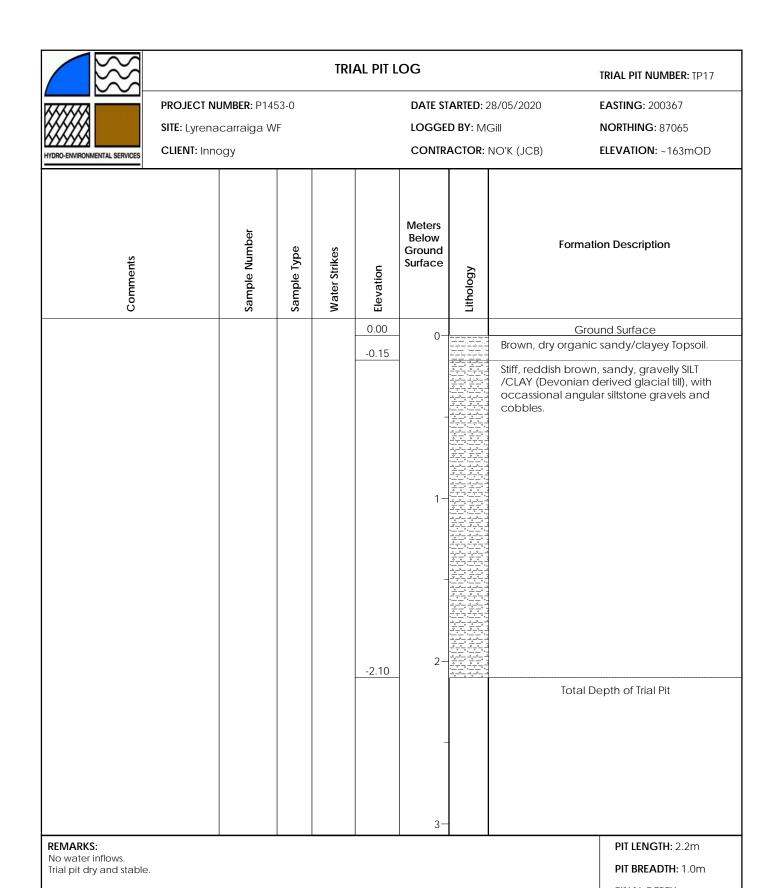
No water inflows.

Trial pit dry and stable.

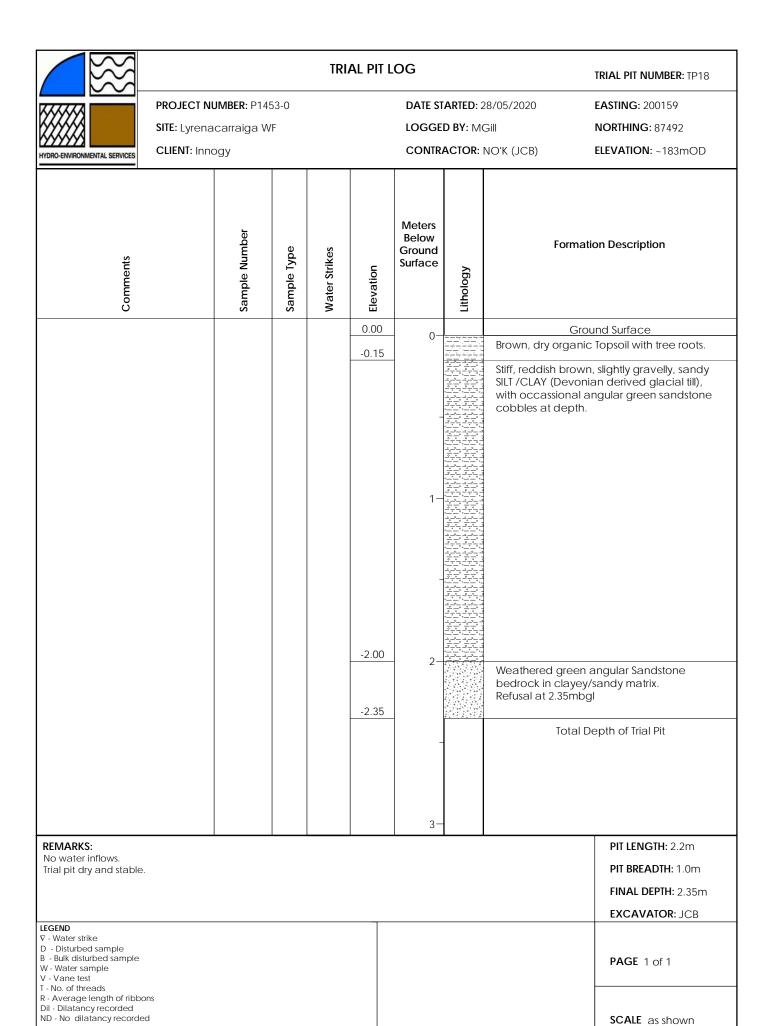








FINAL DEPTH: 2.1m EXCAVATOR: JCB LEGEND 7 - Water strike D - Disturbed sample B - Bulk disturbed sample W - Water sample V - Vane test T - No. of threads R - Average length of ribbons Dil - Dilatancy recorded ND - No dilatancy recorded HYDRO-ENVIRONMENTAL SERVICES 22 Lower Main Street Dungarvan Co. Waterford Tel: 058-44122 Fax: 058-44244 Email: info@hydroenvironmental.ie





DATE STARTED: 28/05/2020

EASTING: 199995

PROJECT NUMBER: P1453-0 SITE: Lyrenacarraiga WF

LOGGED BY: MGill

NORTHING: 87450

TRIAL PIT NUMBER: TP18A

	ogy				CONIK	ACTOR:	NO'K (JCB) ELEVATION : ~187mOD
Comments	Sample Number	Sample Type	Water Strikes	Elevation	Meters Below Ground Surface	Lithology	Formation Description
				0.00	0-		Ground Surface
				-0.15			Brown, dry organic Topsoil.
				-1.20	- 1— - 2—		Stiff, reddish brown, slightly gravelly, sandy SILT /CLAY (Devonian derived glacial till), with occassional angular siltstone cobbles a depth. Stiff, reddish brown, sandy, gravelly, SILT /CLAY (Devonian derived glacial till), with increasing green angular sandstone gravel and cobbles with depth. Possible weathered bedrock from 2.0mbgl.
				-2.20		****	Total Depth of Trial Pit
					3-		
EMARKS: lo water inflows.							PIT LENGTH: 2.2m
rial pit dry and stable.							PIT BREADTH: 1.0m
							FINAL DEPTH: 2.2m
GEND - Water strike							EXCAVATOR: JCB
- Disturbed sample - Bulk disturbed sample - Water sample - Vane test No. of threads - Average length of ribbons							PAGE 1 of 1
- Average length of tibbons I - Dilatancy recorded D - No dilatancy recorded							SCALE as shown



DATE STARTED: 28/05/2020

LOGGED BY: MGill

EASTING: 199657 **NORTHING**: 87270

SITE: Lyrenacarraiga WF

PROJECT NUMBER: P1453-0

TRIAL PIT NUMBER: TP19

NYDRO-ENVIRONMENTAL SERVICES CLIENT: Inno	ogy				CONTRA	ACTOR:	NO'K (JCB) ELEVATION : ~191mOD	
Comments	Sample Number	Sample Type	Water Strikes	Elevation	Meters Below Ground Surface	Lithology	Formation Description	
				0.00	0-		Ground Surface	
				-0.15			Brown, dry organic sandy/clayey Topsoil.	
				-1.50	1-		Stiff, reddish brown, sandy, gravelly SILT /CLAY (Devonian derived glacial till), with occassional angular siltstone gravel and cobbles.	
							Strong, dry, red weathered angular siltstone bedrock in clayey sand matrix.	
Refusal on possible bedrock at 1.8mbgl					-1.80	2-		Possible weathered bedrock from 1.8mbgl. Total Depth of Trial Pit
					3-			
REMARKS: No water inflows. Trial pit dry and stable.							PIT LENGTH: 2.2m PIT BREADTH: 1.0m FINAL DEPTH: 1.8m EXCAVATOR: JCB	
EGEND - Water strike - Disturbed sample - Bulk disturbed sample V Water sample - Vane test - No. of threads							PAGE 1 of 1	
- No. of the ads 8- Average length of ribbons 31- Dilatancy recorded ND - No. dilatancy recorded							SCALE as shown	



DATE STARTED: 28/05/2020

EASTING: 199759

TRIAL PIT NUMBER: TP20

PROJECT NUMBER: P1453-0 SITE: Lyrenacarraiga WF

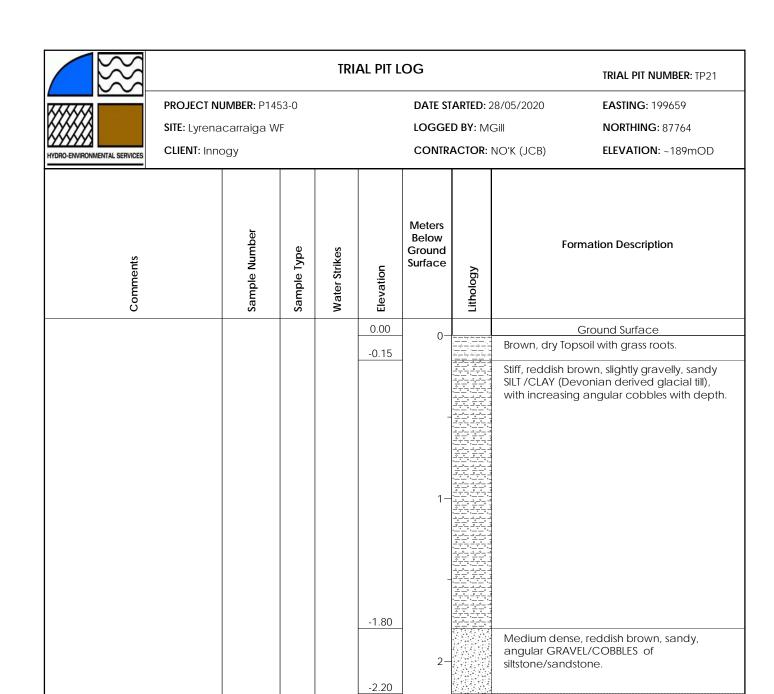
LOGGED BY: MGill

NORTHING: 87789

SCALE

HYDRO-ENVIRONMENTAL SERVICES	CLIENT: Inno	ogy			I	CONTR	ACTOR:	NO'K (JCB)	ELEVATION: ~184mOD
Comments		Sample Number	Sample Type	Water Strikes	Elevation	Meters Below Ground Surface	Lithology	Formati	on Description
water inflow at 2	l.0mbgl			¥	-2.00	1-		Brown, dry soft Top (ground rutted from Stiff, reddish brown (sticky) SILT /CLAY (till), with occassions and cobbles with c	n machinery tracks) , slightly gravelly, sandy Devonian derived glacial al angular siltstone gravels
REMARKS: Water inflow @ 2.0mbg Trial pit stable. Ground rutted from ma conditions (possible see the north)	achinery, soft g)					PIT LENGTH: 2.2m PIT BREADTH: 1.0m FINAL DEPTH: 2.0m EXCAVATOR: JCB
LEGEND 7 - Water strike D - Disturbed sample B - Bulk disturbed sample W - Water sample V - Vane test T - No. of threads R - Average length of ribbo	ns								PAGE 1 of 1

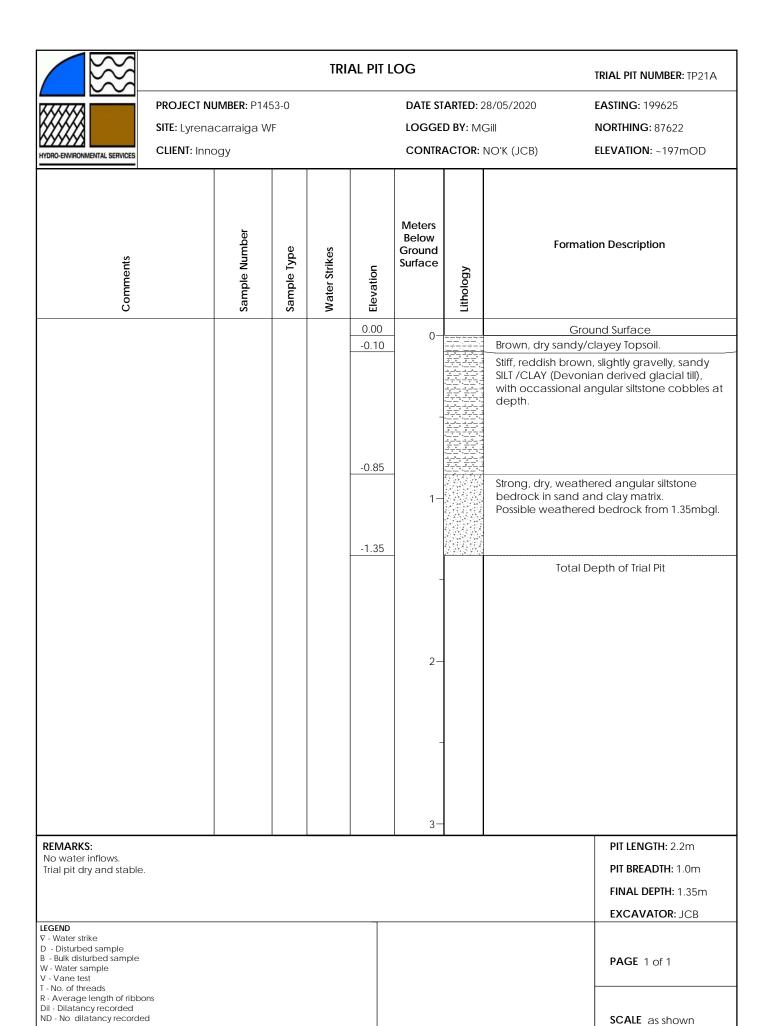
- R Average length of ribbons Dil Dilatancy recorded ND No dilatancy recorded

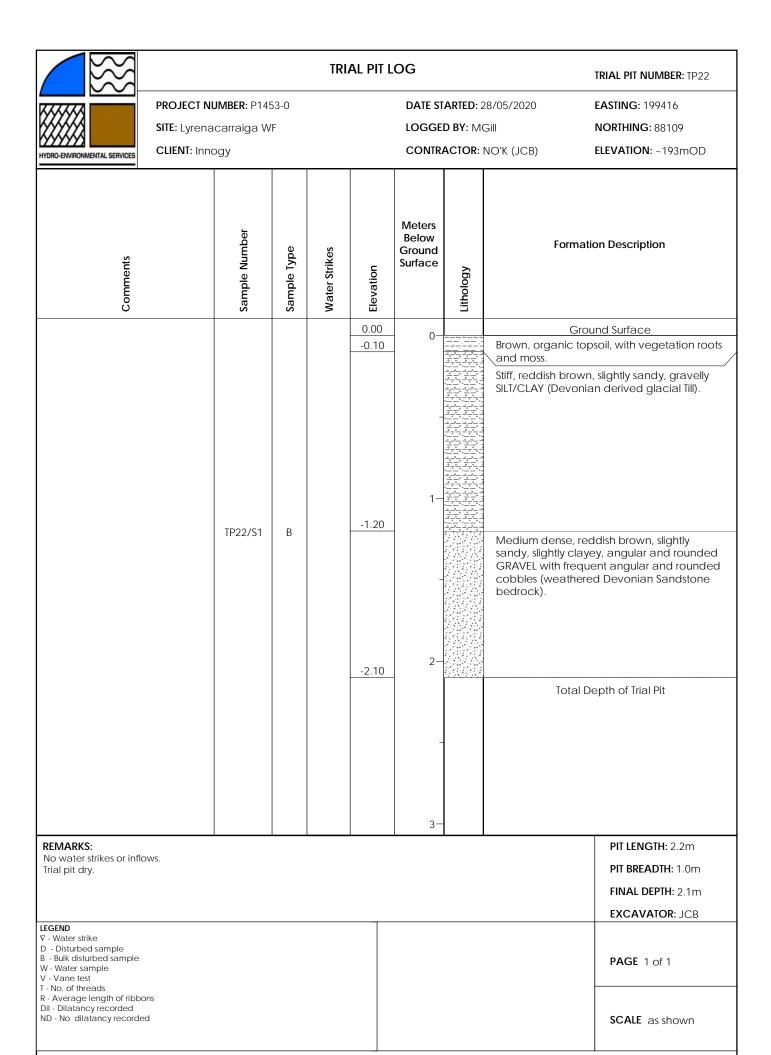


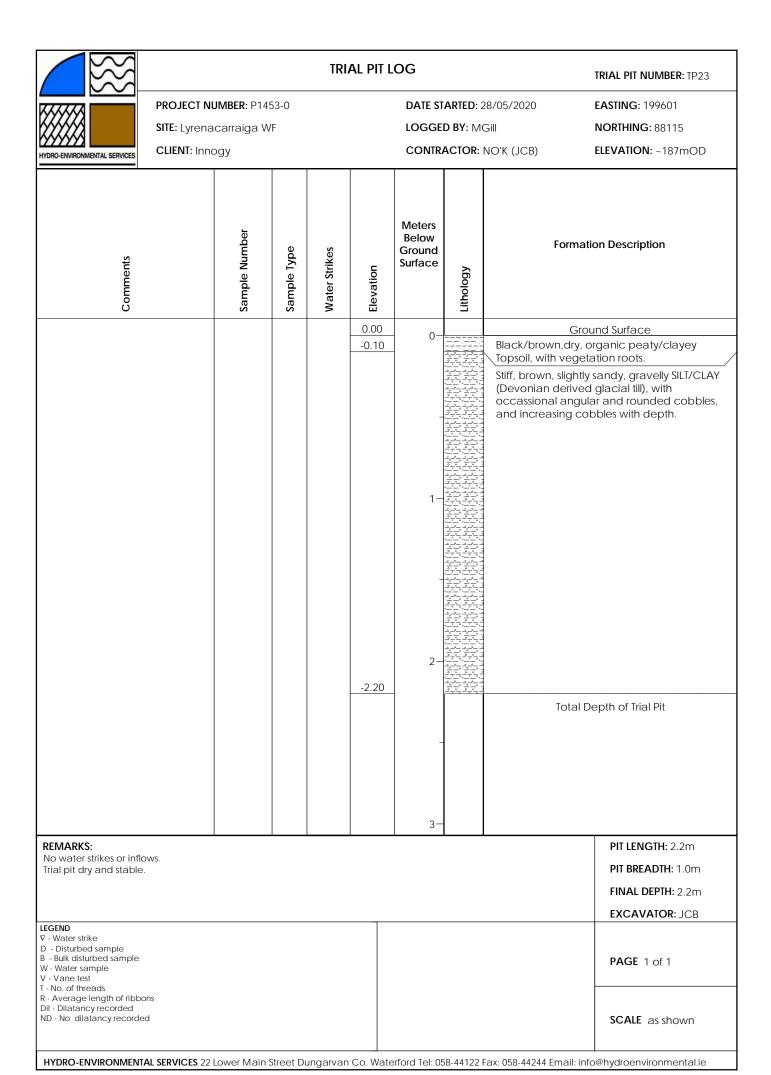
REMARKS: No water strikes or inflows.	PIT LENGTH: 2.2m
Trial pit dry and stable.	PIT BREADTH: 1.0m
	FINAL DEPTH: 2.2m
	EXCAVATOR: JCB
LEGEND	
∇ - Water strike	
D - Disturbed sample	
B - Bulk disturbed sample	PAGE 1 of 1
W - Water sample V - Vane test	
T - No. of threads	
R - Average length of ribbons	
Dil - Dilatancy recorded	
ND - No dilatancy recorded	SCALE as shown

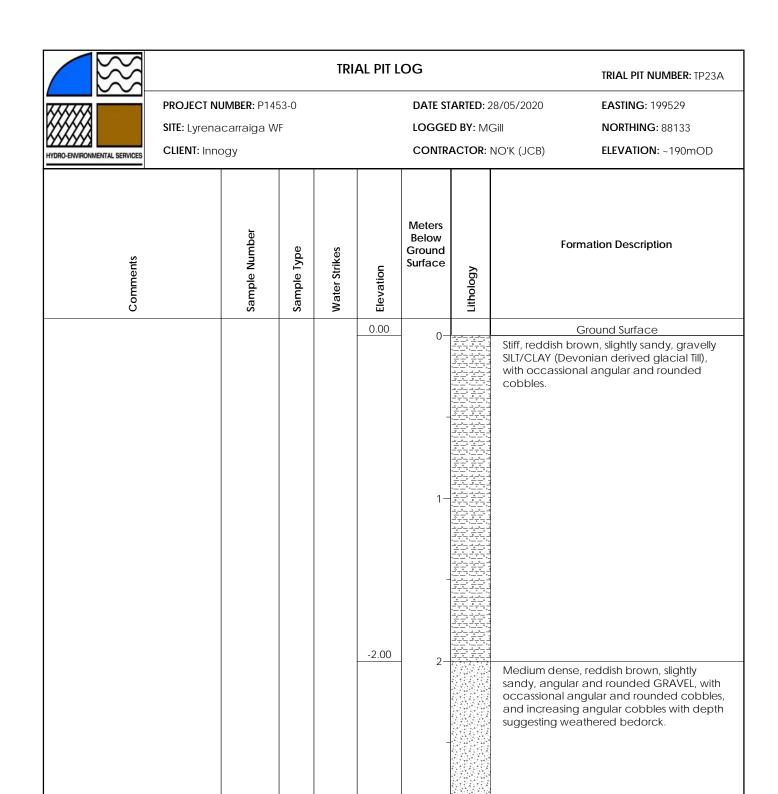
3.

Total Depth of Trial Pit









No water strikes or inflows.
Trial pit dry and stable.

PIT BREADTH: 1.0m

FINAL DEPTH: 2.9m

EXCAVATOR: JCB

LEGEND

V - Water strike
D - Disturbed sample
B - Bulk disturbed sample
W - Water sample
V - Vane test
I - No. of threads
R - Average length of ribbons
Dil - Dilatancy recorded
ND - No dillatancy recorded

SCALE as shown

HYDRO-ENVIRONMENTAL SERVICES 22 Lower Main Street Dungarvan Co. Waterford Tel: 058-44122 Fax: 058-44244 Email: info@hydroenvironmental.ie

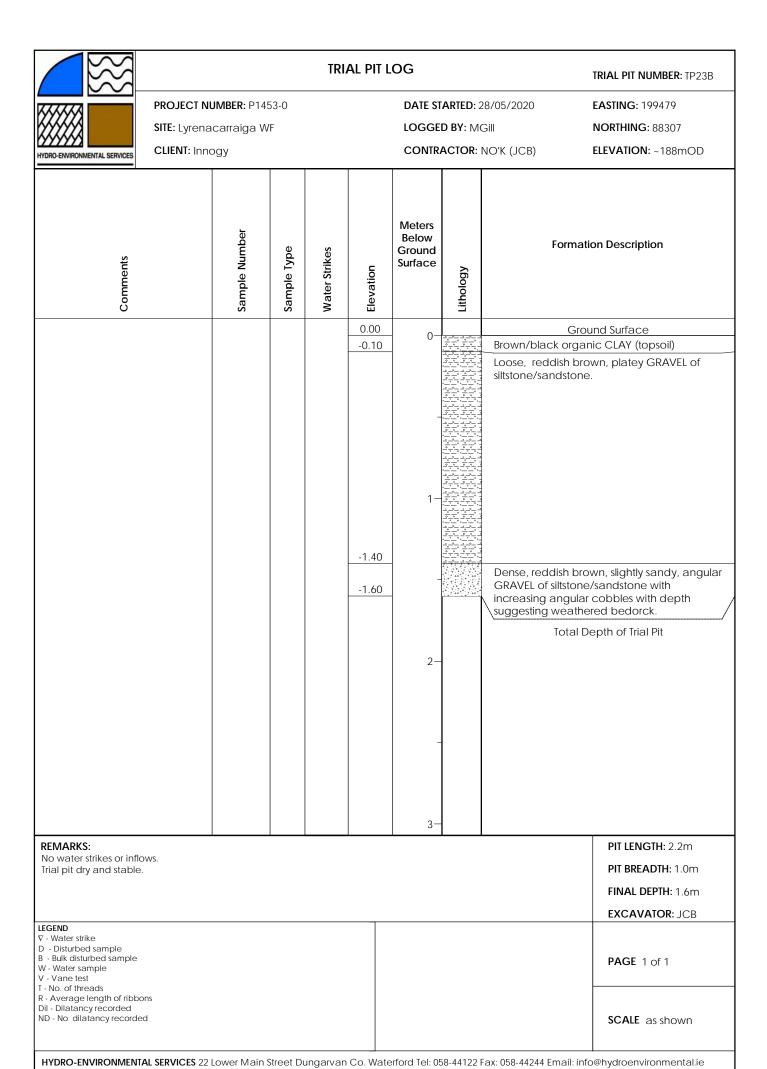
-2.90

REMARKS:

3.

Total Depth of Trial Pit

PIT LENGTH: 2.2m





DATE STARTED: 28/05/2020

LOGGED BY: MGill

EASTING: 199744 **NORTHING**: 88142

PROJECT NUMBER: P1453-0

SITE: Lyrenacarraiga WF

TRIAL PIT NUMBER: TP24

YDRO-ENVIRONMENTAL SERVICES	CLIENT: Inno	ogy			ı	CONTRA	ACTOR: N	IO'K (JCB) ELEVATION: ~18	80mOD
Comments		Sample Number	Sample Type	Water Strikes	Elevation	Meters Below Ground Surface	Lithology	Formation Description	
					0.00	0-		Ground Surface	
					-0.30	1— 1—		Black/brown,dry, organic peaty to vegetation roots. Stiff, reddish brown, slightly sandy, sightly sandy, sight	gravelly al Till). Very I angular
					-2.20	-		Total Depth of Trial Pit	
						3-			
EMARKS: lo water strikes or infi rial pit dry.	ows.				,	•		PIT LENGTH: PIT BREADTH FINAL DEPTH EXCAVATOR	: 1.0m : 2.2m
GEND Water strike - Disturbed sample - Bulk disturbed sample - Water sample - Vane test								PAGE 1 of 1	
No. of threads - Average length of ribb I - Dilatancy recorded D - No dilatancy record								SCALE as sh	own



CONSULTANTS IN ENGINEERING, ENVIRONMENTAL SCIENCE & PLANNING

www.fehilytimoney.ie

Core House Pouladuff Road, Cork, T12 D773, Ireland +353 21 496 4133

Oublin Office

J5 Plaza, North Park Business Park, North Road, Dublin 11, D11 PXTO, Ireland

+353 1 658 3500

Carlow Office

Unit 6, Bagenalstown Industrial Park Bagenalstown, Co. Carlow, R21 XA00, Ireland +353 59 972 3800





